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Ground Investigations Ireland

Hackettstown, Skerries – Southern Greenfield site

DBFL

Ground Investigation Report

July 2020





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1.0 Preamble

On the instructions of DBFL, a site investigation was carried out by Ground Investigations Ireland Ltd., between November 2019 and March 2020 at the site of the proposed new residential development in Hackettstown, Skerries, Co. Dublin.

2.0 Overview

2.1. Background

It is proposed to construct a residential development with associated services, access roads and car parking at the proposed site. The site is currently greenfield agricultural land and is situated on the outskirts of Skerries Village.

The proposed construction is envisaged to consist 2 to 3 story housing and 4 story apartment blocks with conventional or piled foundations and pavement make up with some local excavations for services and plant.

2.2. Purpose and Scope

The purpose of the site investigation was to investigate subsurface conditions utilising a variety of investigative methods in accordance with the project specification. The scope of the work undertaken for this project included the following:

- Visit project site to observe existing conditions
- Carry out 13 No. Trial Pits to a maximum depth of 4.0m BGL
- Carry out 5 No. Soakaways to determine a soil infiltration value to BRE digest 365
- Carry out 25 No. Dynamic Probes to determine soil strength/density characteristics
- Carry out 8 No. Cable Percussion boreholes to a maximum depth of 7.10m BGL
- Carry out 5 No Rotary boreholes to a maximum depth of 17.20m BGL
- Geotechnical & Environmental Laboratory testing
- Report with recommendations

3.0 Subsurface Exploration

3.1. General

During the ground investigation a programme of intrusive investigation specified by the Consulting Engineer was undertaken to determine the sub surface conditions at the proposed site. Regular sampling and in-situ testing was undertaken in the exploratory holes to facilitate the geotechnical descriptions and to enable laboratory testing to be carried out on the soil samples recovered during excavation and drilling.

The procedures used in this site investigation are in accordance with Eurocode 7 Part 2: Ground Investigation and testing (ISEN 1997 – 2:2007) and B.S. 5930:2015.

3.2. Trial Pits

The trial pits were excavated using a 8.5T tracked excavator at the locations shown in the exploratory hole location plan in Appendix 1. The locations were checked using a CAT scan to minimise the potential for encountering services during the excavation. The trial pits were sampled, logged and photographed by a Geotechnical Engineer/Engineering Geologist prior to backfilling with arisings. Notes were made of any services, inclusions, pit stability, groundwater encountered, and the characteristics of the strata encountered and are presented on the trial pit logs which are provided in Appendix 2 of this Report.

3.3. Soakaway Testing

The soakaway testing was carried out in selected trial pits at the locations shown in the exploratory hole location plan in Appendix 1. These pits were carefully excavated and filled with water to assess the infiltration characteristics of the proposed site. The pits were allowed to drain and the drop in water level was recorded over time as required by BRE Digest 365. The pits were logged prior to completing the soakaway test and were backfilled with arising's upon completion. The soakaway test results are provided in Appendix 3 of this Report.

3.4. Dynamic Probing

The dynamic probe tests (DPH) were carried out at the locations shown in the location plan in Appendix 1 in accordance with B.S. 1377: Part 9 1990. The test consists of mechanically driving a cone with a 50kg weight in 100mm intervals and monitoring the number of blows required. An equivalent Standard Penetration Test (SPT) 'N' value may be calculated by dividing the total number of blows over a 300mm drive length by 1.5. The dynamic probe logs are provided in Appendix 4 of this Report.

3.5. Cable Percussion Boreholes

The Cable Percussion Boreholes were drilled using a Dando 2000 drilling rig with regular in-situ testing and sampling undertaken to facilitate the production of geotechnical logs and laboratory testing.

The standard method of boring in soil for site investigation is known as the Cable Percussion method. It consists of using a Shell in non cohesive soils and a clay cutter in cohesive soils, both operated on a wire cable. Very hard soils, boulders and other hard obstructions are broken up by chiselling and the fragments removed with the Shell. Where ground conditions made it necessary, the borehole was lined with 200mm diameter steel casing. While the use of the Cable Percussion method of boring gives the maximum data on soil conditions, some mixing of laminated soil is inevitable. For this reason, thin lenses of granular material may not be noticed. Disturbed samples were taken from the boring tools at suitable depths, so that there is a representative sample at the top of each change in stratum and thereafter at regular intervals down the borehole until the next stratum was encountered. The disturbed samples were then sealed and sent to the laboratory where they were visually examined to confirm the description of the relevant strata. Standard Penetration Tests were carried out in the boreholes. The results of these tests, together with the depths at which the tests were taken are shown on the accompanying borehole records. The test consists

of a thick wall sampler tube, 50mm external diameter, being driven into the soil by a monkey weighing 63.5kg and with a free drop of 760mm. For gravels and glacial till the driving shoe was replaced by a solid 60° cone. The Standard Penetration Test number referred to as the 'N' value is the number of blows required to drive the tube 300mm, after an initial penetration of 150mm. The number gives a guide to the consistency of the soil and can also be used to estimate the relative strength/density at the depth of the test and also to estimate the bearing capacity and compressibility of the soil. The cable percussion borehole logs are provided in Appendix 5 of this Report.

3.6. Rotary Boreholes

The rotary coring was carried out by a track mounted T44 Beretta rig at the locations shown on the location plan in Appendix 1. The rotary boreholes were completed from the ground surface or alternatively, where noted on the individual borehole log, from the base of the cable percussion borehole where a temporary liner was installed to facilitate follow-on rotary coring.

The T44 Beretta is equipped with rubber tracks which allow for short travel on pavement surfaces avoiding any damage to the surface. The T44 Beretta utilises a triple tube core barrel system operated using a wireline drilling process. The outer barrel is rotated by the drill rods and at its lower end, carries the coring bit. The inner barrel is mounted on a swivel so that it does not rotate during the process. The third barrel or liner is placed within the second one to retain the core intact and to preserve as much as possible the fabric of the drilling stratum. The core is cut by the coring bit and passes to the inner liner. The core is brought up to the surface within the inner barrel on a small diameter wire rope or line attached to the "overshoot" recovery tool which is then placed into a core box in order of recovery. A drilling fluid, typically air mist or water flush is passed from the surface through hollow drill rods to the drill bit, and is used to cool the drill bit. Temporary casing is used in some situations to support unstable ground or to seal off fissures or voids. It should be noted that the rotary coring can only achieve limited recovery in overburden, particularly granular or weakly cemented strata due to the flushing medium washing away the cohesive fraction during coring. The recovery achieved, where required is noted on the borehole logs and core photographs are provided to allow assessment of the core recovered. The rotary borehole logs are provided in Appendix 5 of this Report.

3.7. Surveying

The exploratory hole locations have been recorded using a KQ GEO Technologies KQ-M8 System which records the coordinates and elevation of the locations to ITM or Irish National Grid as required by the project specification. The coordinates and elevations are provided on the exploratory hole logs in the appendices of this Report.

3.8. Laboratory Testing

Samples were selected from the exploratory holes for a range of geotechnical and environmental testing to assist in the classification of soils and to provide information for the proposed design.

Environmental & Chemical testing as required by the specification, including the Rilta Suite pH and sulphate testing was carried out by Element Materials Technology Laboratory in the UK. The Rilta suite testing includes both Solid Waste and Leachate Waste Acceptance Criteria.

Geotechnical testing consisting of moisture content, Atterberg limits, Particle Size Distribution (PSD), hydrometer, California Bearing Ratio (CBR), tests were carried out in NMTL's Geotechnical Laboratory in Carlow.

Rock strength testing including Point Load (I_{s50}) testing was carried out in NMTL's Geotechnical Laboratory in Carlow. The results of the laboratory testing are included in Appendix 6 of this Report.

4.0 Ground Conditions

4.1 General

The ground conditions encountered during the investigation are summarised below with reference to insitu and laboratory test results. The full details of the strata encountered during the ground investigation are provided in the exploratory hole logs included in the appendices of this report.

The sequence of strata encountered were variable across the site but generally comprised;

- Topsoil
- Made Ground
- Cohesive Deposits
- Granular Deposits
- Bedrock

TOPSOIL: Topsoil was encountered in all of the exploratory holes on the site and was present to a maximum depth of 0.5m BGL.

MADE GROUND: Made Ground deposits were not prominent in the southern greenfield part of the site. However it was encountered in TP17 and TP10 to a maximum depth of 1m BGL that was described generally as *brown slightly sandy slightly gravelly Clay*. These deposits had occasional or some cobble content where noted on the exploratory hole logs. It should be noted that TP13 is located to the north of the site boundary and also encountered Made ground to a depth of 2.60m BGL that consisted of clayey gravelly Sand or sandy gravelly Clay.

COHESIVE DEPOSITS: Cohesive deposits were encountered beneath the topsoil and were described typically as *brown sandy gravelly CLAY / silty CLAY with occasional cobbles and boulders*. The secondary sand and gravel constituents varied across the site and with depth, with granular lenses occasionally present in the glacial till matrix. These deposits had some, occasional or frequent cobble and boulder content where noted on the exploratory hole logs.

The strength of the cohesive deposits varied across the site but generally increased with depth and was typically soft to depths of between 1.7 and 3.4m BGL overlying firm, firm to stiff or stiff in the majority of the exploratory holes.

GRANULAR DEPOSITS: The granular deposits were encountered within the cohesive deposits and were typically described as *grey or brown clayey sandy sub rounded to sub angular fine to coarse GRAVEL or gravelly fine to coarse SAND*. The secondary sand/gravel and silt/clay constituents varied across the site and with depth while occasional or frequent cobble and boulder content was also present where noted on the exploratory hole logs.

Based on the SPT N values the deposits are typically medium dense and become dense with depth although loose deposits were recorded in places. It should be noted that some of the trial pits where granular deposits or groundwater were encountered, experienced instability. This was described either as side wall spalling or as side wall collapse in the remarks section at the base of the trial pit logs. Groundwater strikes were noted in the majority of the boreholes generally on encountering or within the granular deposits.

BEDROCK: The rotary core boreholes recovered medium strong to strong grey fine grained LIMESTONE. Rare to some Calcite veins were noted during logging which are typically present within the Limestone. The depth to medium strong to strong limestone rock varies from 9.8m BGL in BH02 to 13.2m BGL in BH09. Limestone was not encountered in RC10 before 17.2m BGL but hard pinkish grey gravelly CLAY which is described as possible weathered rock was encountered at a depth of 14.6m BGL. The total core recovery is good, typically 100% with some of the uppermost runs dropping to 80 or 90%. The SCR and RQD both are relatively good although some areas are recovered as non-intact.

4.2. Insitu Strength Testing

The correlated DPH blow counts indicate that the overburden deposits on the site are soft or soft to firm / loose to depths of 1.1m to 3.4m BGL and become firm or firm to stiff / medium dense with depth.

Some areas show deeper soft materials particularly the western side of the site where DP08, DP09, DP04, DP01, BH07 and BH03 are located. These locations coincide with a depression at the surface and indicate soft or soft to firm cohesive material or loose granular materials to depths of between 2.00m and 3.40m BGL.

4.3. Groundwater

Groundwater strikes are noted on the exploratory hole logs where they occurred and where possible drilling was suspended for twenty minutes to allow the subsequent rise in groundwater to be recorded. We would point out that these exploratory holes did not remain open for sufficiently long periods of time to establish the hydrogeological regime and groundwater levels would be expected to vary with the tide, time of year, rainfall, nearby construction and other factors. For this reason, standpipes were installed in BH03, BH05A, BH07, and RC09 to allow the equilibrium groundwater level to be determined. The groundwater monitoring is included in Appendix 7 of this Report.

4.4. Laboratory Testing

4.4.1. Geotechnical Laboratory Testing

The geotechnical testing carried out on soil samples recovered generally confirm the descriptions on the logs with the primary constituent of the cohesive deposits found to be a CLAY of low plasticity. The Particle Size Distribution tests confirm that generally the cohesive deposits are well-graded with percentages of sands and gravels ranging between 13% and 45% generally with fines contents of 30 to 50%.

The Particle Size Distribution tests confirm that generally the granular deposits are well-graded with percentages of sands/gravels and silt/clay typically between 25% and 35% with a gravel/sand content of typically 60% to 70%.

The CBR testing on remoulded samples from the site gave results ranging between 0.08% and 1.45% for the cohesive deposits.

4.4.1. Chemical Laboratory Testing

The pH and sulphate testing carried out indicate that pH results are near neutral and that the water soluble sulphate results is low when compared to the guideline values from BRE Special Digest 1:2005. The samples tested classify the soil as a Design Sulphate Level DS-1.

4.4.1. Environmental Laboratory Testing

A number of samples were analysed for a suite of parameters which allows for the assessment of the sampled material in terms of total pollutant content for classification of materials as *hazardous* or *non-hazardous*. The suite also allows for the assessment of the sampled material in terms of suitability for placement at licenced landfills (inert, stable non-reactive, hazardous etc.). The parameter list for the suite includes analysis of the solid samples for arsenic, barium, cadmium, chromium, copper, cyanide, lead, nickel, mercury, zinc, speciated aliphatic and aromatic petroleum hydrocarbons, pH, sulphate, sulphide, moisture content, soil organic matter and an asbestos screen.

The suite also includes those parameters specified in the EU Council Decision establishing criteria for the acceptance of waste at Landfills (Council Decision 2003/33/EC), which for the solid samples are total organic carbon (TOC), speciated aliphatic and aromatic petroleum hydrocarbons, BTEX, phenol, polychlorinated biphenyls (PCB) and PAH.

As part of the suite a leachate is generated from the solid sample which is analysed for antimony, arsenic, barium, cadmium, chromium, copper, lead, mercury, molybdenum, nickel, selenium, zinc, chloride, fluoride, soluble sulphate, sulphide, phenols, dissolved organic carbon (DOC) and total dissolved solids (TDS).

While the laboratory report provides a comparison with the waste acceptance criteria limits it does not provide a waste classification of the material sampled nor does it comment on any potentially hazardous properties of the materials tested. The possibility for contamination, not revealed by the testing undertaken should be borne in mind particularly where Made Ground deposits are present or the previous site use or location indicate a risk of environmental variation.

The waste classification report is included under the cover of a sperate report by Ground Investigations Ireland.

4.4.1.Rock Laboratory Testing

The point load testing carried out on samples recovered from the boreholes gave I_{s50} values ranging between 1.4 to 5.4 MPa. The I_{s50} results correlate to the UCS values using a factor of approximately 20, giving values of 28 MPa and 108 MPa. These results correlate to the strength descriptions ranging between of Medium Strong to Very Strong and confirming the variability of this stratum and the descriptions on the logs. The average of the correlated values from the point loading suggest the rock is typically Strong. The results from the completed laboratory testing is included in Appendix 6 of this report.

5.0 Recommendations & Conclusions

5.1. General

The recommendations given and opinions expressed in this report are based on the findings as detailed in the exploratory hole records. Where an opinion is expressed on the material between exploratory hole locations, this is for guidance only and no liability can be accepted for its accuracy. No responsibility can be accepted for conditions which have not been revealed by the exploratory holes. Limited information has been provided at the ground investigation stage and any designs based on the recommendations or conclusions should be completed in accordance with the current design codes, taking into account the variation and the specific details contained within the exploratory hole logs.

5.2. Foundations

The ground conditions on the southern greenfield part of the site are variable including soft compressible soils. Due to the variation in conditions, depths have been provided in table 1 at each of the investigation locations where an allowable bearing capacity of 100 kN/m² and 200kN/m² is achievable for conventional strip or pad foundations on the firm to stiff / stiff cohesive deposits or medium dense granular deposits. The possibility for variation in the depth of the soft or loose deposits in the vicinity of these foundations should be considered and foundation inspections should be carried out. Any areas where suitable bearing strata is deep or soft spots encountered at the proposed foundation depths should be excavated and replaced with lean mix concrete.

Table 1 - Allowable Bearing Capacities at Investigation Locations on Green Field Site

		Allowable Bearing Capacities (ABC) kN/m ²					
Probe / Borehole	100 Kn /M2 ABC	200 Kn /M2 ABC	Comment	Probe / Borehole	100 Kn /M2 ABC	200 Kn /M2 ABC	Comment
No.	Depth mBGL	Depth m BGL		No.	Depth mBGL	Depth m BGL	
DP01	2.7	3.7		DP18	1.8	2.3	
DP02	1.5	3.3		DP19	2.4	2.9	
DP03	1.1	2.2		DP20	1.7	2.4	
DP04	2.8	3.6		DP21	1.8	2.6	
DP05A	1.3	N/A	Refusal at 1.4	DP22	1.2	1.5	
DP06	1.7	2.4		DP23	1.7	1.9	
DP07	1.4	N/A		DP24	2.5	2.7	
DP08	3.6	4.4		DP25	1.6	2.0	
DP09	2.1	2.6		BH01	5.7	5.7	Bridge culvert, See section 5.3
DP10	1.8	3.2		BH02	1.7	3.8	
DP11	2.1	5.0		BH03	3.0	4.0	
DP12	1.2	1.7		BH04	2.0	2.0	
DP13	1.2	2.2		BH05	2.0	2.0	
DP14	1.7	2.2		BH05A	2.0	2.0	
DP15	2.5	2.9		BH06	2.1	2.1	
DP16	1.7	2.3		BH07	3.4	3.4	
DP17	2.0	2.0					
In areas with deeper soft / loose ground conditions and/or higher loadings, piling may be more economical, see below							

Where possible a ground bearing floor slab is recommended to be based on the firm or firm to stiff / cohesive deposits or medium dense granular deposits with an appropriate depth of compacted hardcore specified by the consulting engineer and in accordance with the limits and guidelines in SR21:2014+A1:2016 and/or NRA SRW CL808 Type E granular stone fill. Where the depth of Made Ground/Soft deposits exceeds 0.9m then suspended floor slabs should be considered.

Due to the variation in ground conditions we would recommend that all the foundations from individual buildings are founded in the same stratum to avoid differential settlement. This is particularly important to consider for longer sections of connected buildings that are more likely to cross areas of differing ground conditions i.e. apartment blocks, terraced housing.

Due to the presence of soft and compressible cohesive or loose granular deposits beneath the footprint of the proposed structures on some parts of the site conventional foundations may not be practical. In these areas piled foundations may be more economically advantageous for the proposed buildings particularly where higher loading is anticipated at the location of the proposed 3 or 4 story buildings. The deepest depths shown in the table are due to the increased depth of softer/loose materials and give an indication of the areas of the site where piling may be required. As mentioned in section 4.2 of this report the western part of the greenfield site is one area where the probes and boreholes indicate deeper soft/loose materials. The type, size and depth of the pile foundations should be confirmed by a specialist piling contractor based on the loading from the proposed building. The floor slab is recommended be suspended and also supported on the building piles. Negative skin friction from the very soft cohesive deposits should be considered in the pile design due to the possibility of loading from working platforms or the adjacent pavement make up. The slope leading to the stream should also be considered when determining foundation types as loading and excavation could cause instability meaning piled foundations may be more suitable if buildings with higher loadings are proposed in this area.

The recommended bearing capacity that can be achieved on the limestone rock would be in excess of 1000 kN/m².

When the location and loading of buildings has been finalised for the proposed development it may be economically advantageous to undertake additional investigation to determine the extents of softer ground and the types of foundation that could be used for individual structures.

5.3. Foundations for Bridge Culvert

Due to the presence of variable made ground beneath the footprint of the northside of the proposed bridge in the location of BH01 and soft materials on the south site of the bridge at the location of BH02, piled foundations may be required, However when proposed bridge culvert loadings are finalised foundation type can be determined.

The type, size and depth of the pile foundations should be confirmed by a specialist piling contractor based on the loading from the proposed building.

Negative skin friction from the very soft cohesive deposits should be considered in the pile design due to the possibility of loading from working platforms or the adjacent pavement make up.

5.4. Groundwater

Bearing capacities provided within the report take into account the groundwater level. The design ground water level for the site are provided in the table below water levels were generally higher towards the northern part of the site nearer to the stream so levels in the eastern and western parts of the site are based on the highest reading recorded. Piezometers were not installed in all areas of the site so water levels provided are based on where water was encounter during excavation of trial pits and boreholes and may not represent standing water levels.

Area	Design Groundwater level	Investigation locations considered	Notes
Western part of site (Purple area*)	1.0m BGL	Piezo in BH03 and BH07	Based on piezometer readings
Eastern part of site (Green area*)	1.5m BGL	Piezo in BH05, RC09, BH06	Based on piezo readings in BH05 and BH09 and borehole records in BH06
*Water levels based on reading taken in summer months, levels would be expected to vary with the tide, time of year, rainfall, nearby construction and other factors.			
** Figure 2 in appendix 1 shows DBFL designated areas on site.			

The groundwater monitoring is included in Appendix 7 of this Report.

5.5. Chemical Testing Results

The pH and sulphate testing completed on samples recovered from the exploratory holes across the northern green field and southern brown field parts of the site indicates the pH results are near neutral and the sulphate results are low, when compared to the guideline values from BRE Special Digest 1:2005. No special precautions are required for concrete foundations to prevent sulphate attack. The samples tested were below the limits of DS1 in the BRE Special Digest 1:2005.

5.6. External Pavements

The proposed pavements are recommended to be designed in accordance with the CBR test results included in the Appendixes of this Report. The low CBR test results from the southern part of the site indicate that a capping layer or a sufficient depth of crushed stone fill may be required. The CBR values gained from the northern part of the site were higher but should be used with caution due to the likely variation within the made ground. Plate bearing tests are recommended at the time of construction to verify the design assumptions for the proposed pavement make up and to verify adequate compaction has been achieved.

The use of a geogrid and separation membrane may improve the performance of the proposed pavement and enable a more economical pavement design to be achieved, a specialist supplier is recommended to advise of the required strength, depth and type of geotextile for the proposed design.

5.7. Excavations

Short term temporary excavations in the cohesive deposits will remain stable for a limited time only and will require to be appropriately battered or the sides supported if the excavation is below 1.25m BGL or is required to permit man entry.

Excavations in the Made Ground or soft Cohesive Deposits will require to be appropriately battered or the sides supported due to the low strength of these deposits.

Any excavations which penetrate the granular deposits will require to be appropriately battered or the sides supported and are likely to require dewatering due to the groundwater seepages noted in the exploratory hole logs in the Appendices of this Report.

The groundwater and stability noted on the trial pit logs should be consulted when determining the most appropriate construction methods for excavations.

Excavations in the upper cohesive and granular deposits are expected to be excavatable with conventional excavation equipment. Due to the depth that rock was encountered excavations in rock are not anticipated to be required.

5.8. Soakaway Design

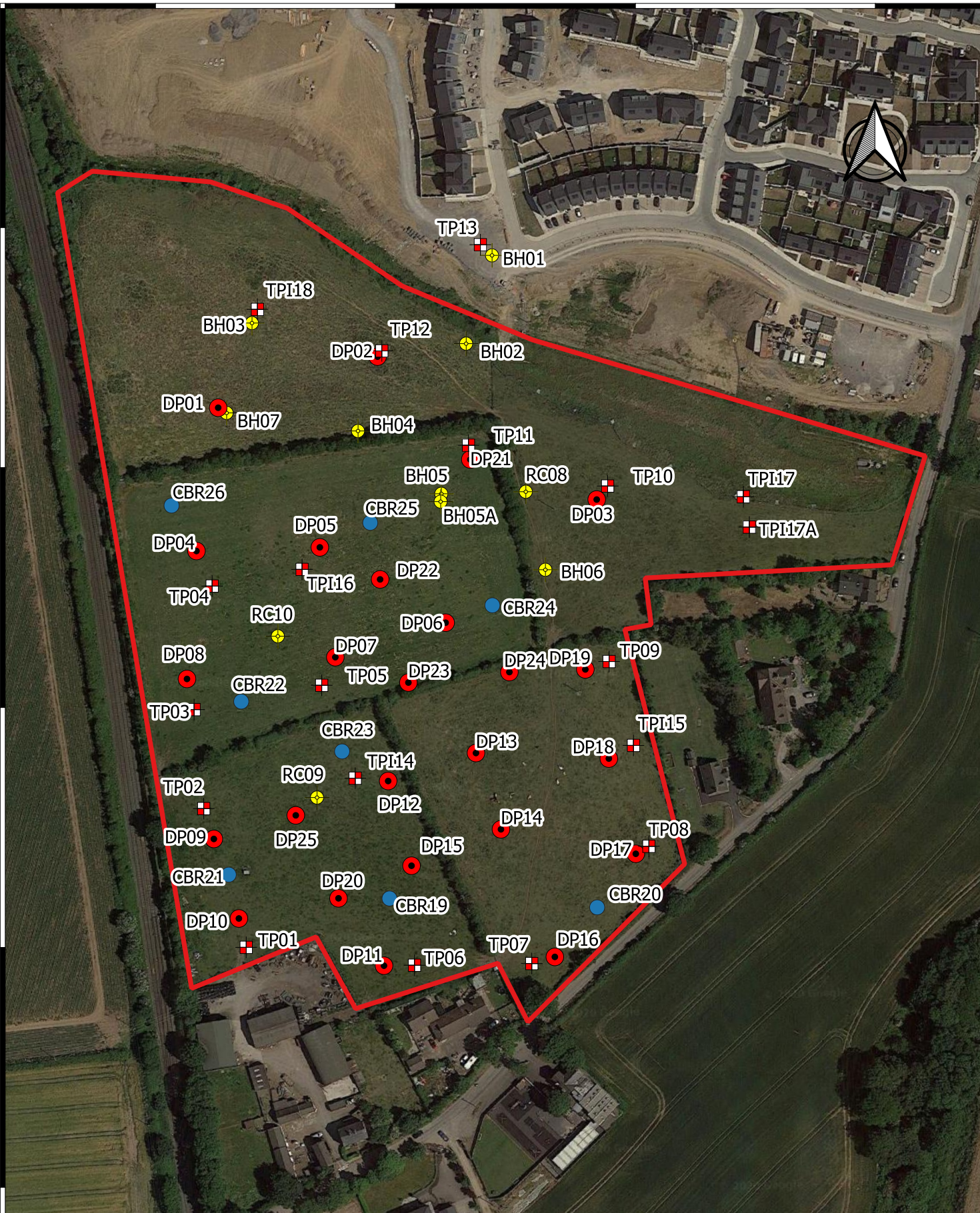
An infiltration rate of $f=8.2 \times 10^{-6}$ m/s were calculated for the soakaway location TPI16. At the locations of TPI14, TPI15, TPI17A, and TPI18 the water level dropped too slowly to allow calculation of 'f' the soil infiltration rate. These locations are therefore not recommended as suitable for soakaway design and construction.

The recommendations provided in this report should be verified in the design of the proposed buildings, using the full details of the loading conditions and taking into consideration the allowable tolerable settlements/movements that the building can accommodate. The founding strata should be inspected and verified by a suitably qualified engineer prior to construction of the building foundations.

APPENDIX 1 - Site Location Plan



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0 20 40 60 m








Project Title:
Hackettstown, Skerries

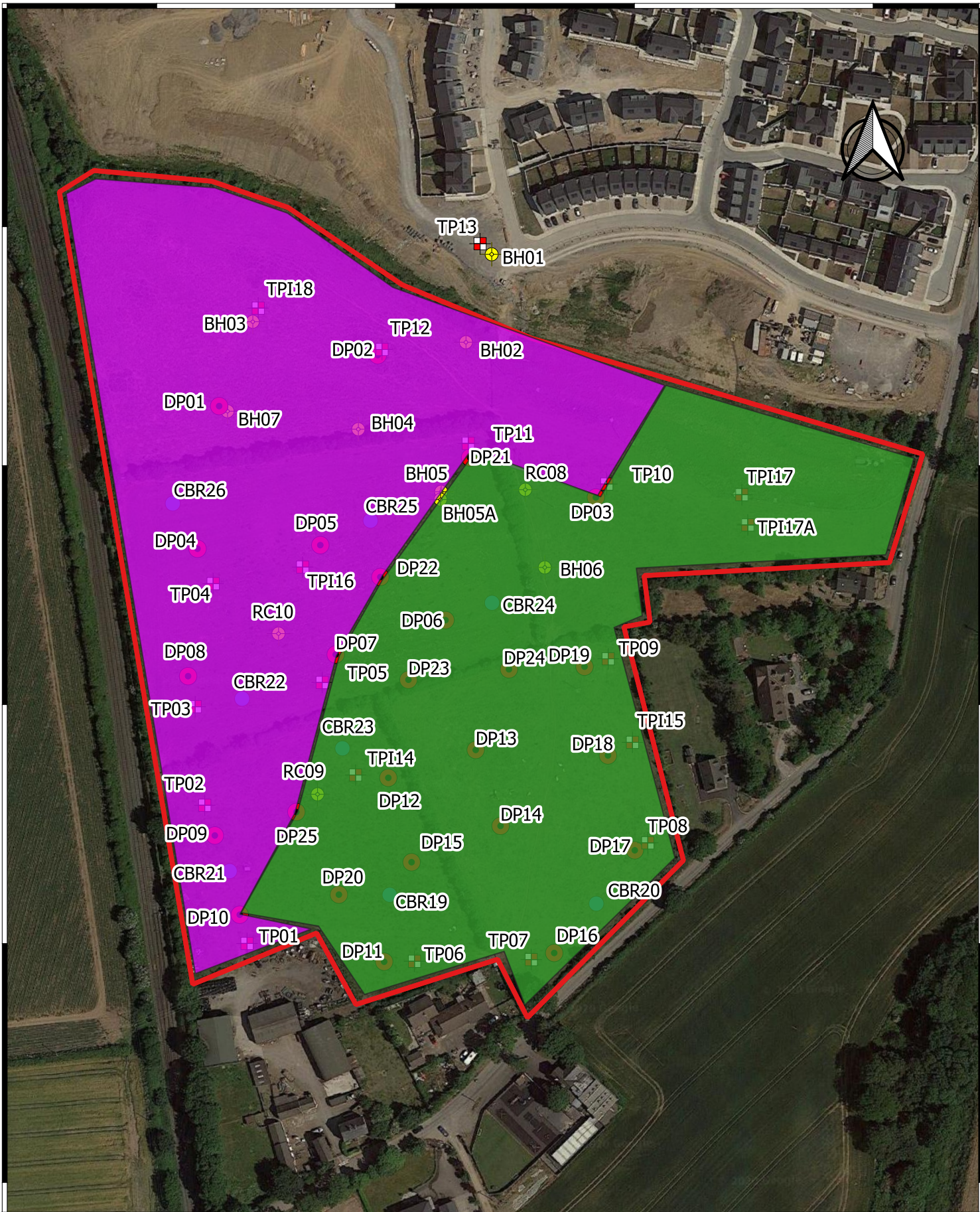
Drawing Title:
Figure 1 Investigation Location

GII Project Reference:
9225-11-19

Drawn By:
MS

Date:
03/07/2020

-  Borehole
-  Trial Pit
-  Dynamic Probe
-  CBR
-  Indicative Site Boundary



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Client:



0 20 40 60 m



Project Title:
Hackettstown, Skerries

Drawing Title:
Figure 2 Site zones

GII Project Reference:
9225-11-19

Drawn By:
MS

Date:
03/07/2020

- Borehole
- Trial Pit
- Dynamic Probe
- CBR
- Indicative Site Boundary

APPENDIX 2 – Trial Pit Records





Machine : 8.5T Excavator Method : Trial Pit	Dimensions 1.0m x 2.5m x 3.40m	Ground Level (mOD) 24.45	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724937.8 E 758999.6 N	Dates 28/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.70	B			24.25	(0.20)	Brown slightly sandy slightly gravelly TOPSOIL.		
					0.20	Soft to firm orange brown slightly sandy slightly gravelly CLAY. Gravel is sub-angular to sub rounded fine to coarse. Sand is fine to coarse.		
1.50	B		Water strike(1) at 1.50m.	23.25	(1.00)			
					1.20	Firm brown slightly sandy slightly gravelly CLAY with some cobbles and occasional boulders. (Band of clayey gravel at 1.5m-2.30m). Gravel is sub-angular to sub-rounded fine to coarse. Sand is fine to coarse.		
2.70	B			22.65	(0.60)			V1
					1.80	Firm to stiff brown slightly sandy slightly gravelly CLAY with some cobbles and occasional boulders. (Band of clayey gravel at 1.5m-2.30m). Gravel is sub-angular to sub-rounded fine to coarse. Sand is fine to coarse.		
					2.30	Firm to stiff reddish brown slightly sandy slightly gravelly silty CLAY with occasional cobbles and boulders. Gravel is sub-angular to sub-rounded fine to coarse. Sand is fine to coarse.		
				21.35	(0.80)			
					3.10	Stiff reddish brown slightly sandy slightly gravelly CLAY with occasional cobbles and boulders. Gravel is sub-angular to sub-rounded fine to coarse. Sand is fine to coarse.		
				21.05	3.40	Complete at 3.40m		

Plan .	Remarks Trial pit side wall collapse from 0.5-3.0m Trial pit complete at 3.40m due to collapse. Trial pit backfilled on completion. Groundwater seepage from 1.5-2.0m	
		Scale (approx) 1:25



Machine : 8.5T Excavator Method : Trial Pit	Dimensions 10m x 2.5m x 3.6m	Ground Level (mOD) 23.91	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724920.1 E 759057.4 N	Dates 28/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B1			23.61	(0.30)	Brown slightly gravelly slightly sandy TOPSOIL.		
					(0.70)	Soft to firm brown slightly sandy slightly gravelly CLAY with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
1.20	B2			22.91	(1.00)	Soft to firm brown slightly sandy slightly gravelly CLAY with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
					(2.00)	Firm reddish brown slightly sandy slightly gravelly CLAY with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded, fine to coarse.		
2.80	B3			21.41	(2.50)	Stiff reddish brown slightly sandy slightly gravelly CLAY with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded, fine to coarse.		
					(3.60)	Complete at 3.60m		
				20.31				
								Water strike(1) at 3.30m.

Plan	Remarks		
	<p>Moderate water inflow from 3.30m. Trial pit terminated due to collapse and water inflow. Trial pit sidewall collapse from all sides. Trial pit backfilled on completion.</p>		
	Scale (approx)	Logged By	Figure No.
	1:25	MS	9225-11-19.TP02



Machine : 8.5T Excavator Method : Trial Pit	Dimensions 1m x 2.5m x 3.7m	Ground Level (mOD) 23.76	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724916.1 E 759098.7 N	Dates 28/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			23.46	0.30	Brown slightly sandy slightly gravelly TOPSOIL.		
					0.40	Soft brown slightly sandy slightly gravelly silty CLAY. Gravel is sub-angular to sub-rounded fine to coarse. Sand is fine to coarse.		
1.00	B			23.06	0.70	Soft orangey brown slightly sandy slightly gravelly CLAY. Gravel is sub-angular to sub-rounded fine to coarse. Sand is fine to coarse.		
1.20	B				(1.30)			
2.00	B			21.76	2.00	Soft slightly gravelly sandy CLAY with occasional sub-angular to rounded cobbles. Gravel is sub-angular to rounded fine to coarse. Sand is fine to coarse.		
					(0.50)			
				21.26	2.50	Loose red brown very clayey gravelly fine to coarse SAND with occasional sub-angular to rounded cobbles. Gravel is sub-angular to rounded fine to coarse.		
3.00	B				(0.90)			
				20.36	3.40	Medium dense red brown very clayey gravelly fine to coarse SAND with occasional sub-angular to rounded cobbles. Gravel is sub-angular to rounded fine to coarse.		
					(0.30)			
				20.06	3.70	Complete at 3.70m		

Plan	Remarks							
	<p>Trial pit side wall collapse. Trial pit terminated due to collapse. Trial pit backfilled on completion. No groundwater encountered during excavation.</p>							
	Scale (approx)			Logged By		Figure No.		
	1:25			MS		9225-11-19.TP03		



Machine : 8.5T Excavator Method : Trial Pit		Dimensions 1.0m x 2.5m x 3.30m	Ground Level (mOD) 23.11	Client DBFL	Job Number 9225-11-19
		Location (dGPS) 724923.6 E 759150.2 N	Dates 29/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B		Water strike(1) at 0.90m.	22.91	(0.20)	TOPSOIL.		
					0.20	Soft brown sandy gravelly silty CLAY with occasional sub-angular to sub-rounded cobbles and occasional sandy lenses. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
1.50	B		Water strike(1) at 0.90m.	22.01	(0.90)			∇1
					1.10	Soft to firm brown sandy gravelly silty CLAY with occasional sub-angular to sub-rounded cobbles and occasional sandy lenses. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
2.50	B		Water strike(2) at 3.30m.	20.31	(1.70)			
					2.80	Firm to stiff brown sandy gravelly silty CLAY with occasional sub-angular to sub-rounded cobbles and occasional sandy lenses. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
				19.81	3.30	Complete at 3.30m		∇2

Plan .	Remarks Groundwater encountered at 0.9m. Steady trickle. Groundwater encountered at 3.30m. Trial pit side wall collapse. Trial pit terminated due to instability. Trial pit backfilled on completion.	
		Scale (approx) 1:25



Machine : 8.5T Excavator Method : Trial Pit	Dimensions 1.0m x 2.5m x 3.5m	Ground Level (mOD) 24.89	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724969.3 E 759108.8 N	Dates 28/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.60	B			24.49	(0.40)	Dark brown slightly sandy slightly gravelly TOPSOIL		
					0.40	Loose reddish brown clayey gravelly fine to coarse SAND. Gravel is sub-angular to sub-rounded, fine to coarse.		
1.50	B			23.59	(0.90)			
					1.30	Firm to stiff brown slightly sandy slightly gravelly CLAY with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
2.50	B			22.69	(1.30)			
					2.20	Yellow brown clayey gravelly fine to coarse SAND with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded fine to coarse. Sand is predominantly fine to medium.		
3.50	B			21.39	3.50	Complete at 3.50m		

Plan .	Remarks Trial pit terminated due to difficult excavation. No groundwater encountered during excavation. Trial pit stable. Trial pit backfilled on completion.					
	<table border="1"> <tr> <td>Scale (approx)</td> <td>Logged By</td> <td>Figure No.</td> </tr> <tr> <td>1:25</td> <td>MS</td> <td>9225-11-19.TP05</td> </tr> </table>	Scale (approx)	Logged By	Figure No.	1:25	MS
Scale (approx)	Logged By	Figure No.				
1:25	MS	9225-11-19.TP05				



Machine : 8.5T Excavator Method : Trial Pit	Dimensions	Ground Level (mOD) 24.90	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725007.9 E 758992.1 N	Dates 28/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			24.70	(0.20)	Brown slightly sandy slightly gravelly TOPSOIL.		
					0.20	Soft to firm brown slightly gravelly very sandy CLAY. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
1.50	B		Water strike(1) at 1.00m. Water strike(2) at 1.50m.	24.00	(0.70)			∇1
					0.90	Soft to firm slightly gravelly sandy CLAY with occasional sub-angular to sub-rounded cobbles and occasional lenses of yellow brown fine sand. Gravel is sub-angular to sub-rounded, fine to coarse.		
2.50	B			23.10	1.80	Firm slightly gravelly sandy CLAY with occasional sub-angular to sub-rounded cobbles and occasional lenses of yellow brown fine sand. Gravel is sub-angular to sub-rounded, fine to coarse.		∇2
					(0.40)			
					2.20	Firm to Stiff slightly gravelly sandy CLAY with occasional sub-angular to sub-rounded cobbles and occasional lenses of yellow brown fine sand. Gravel is sub-angular to sub-rounded, fine to coarse.		
				22.70	(0.30)			
				22.40	2.50	Complete at 2.50m		

Plan .	Remarks Moderate groundwater inflow from 1.0m and 1.5m. Collapse from all sides of trial pit. Trial pit terminated due to collapse. Trial pit backfilled on completion.	
		Scale (approx) 1:25



Machine : 8.5T Excavator Method : Trial Pit	Dimensions 1.0m x 2.5m x 3.4m	Ground Level (mOD) 25.04	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725056.8 E 758992.8 N	Dates 28/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			24.74	(0.30) 0.30	Brown slightly sandy slightly gravelly TOPSOIL.		
1.20	B			23.54	(1.20) 1.50	Soft to firm orange brown slightly sandy slightly gravelly CLAY. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
2.20	B		Water strike(1) at 1.50m.	22.74	(0.80) 2.30	Firm orange brown slightly sandy slightly gravelly CLAY. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		▽1
3.00	B			22.04	(0.70) 3.00	Stiff brown slightly sandy slightly gravelly CLAY with occasional sub-angular to sub-rounded cobbles and boulders. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
				21.64	(0.40) 3.40	Very stiff brown slightly sandy slightly gravelly CLAY with occasional sub-angular to sub-rounded cobbles and boulders. Gravel is sub-angular to sub-rounded, fine to coarse.		
						Complete at 3.40m		

Plan .	Remarks Groundwater seepage from 1.5m to 2.6m from all faces. Minor trial pit side wall collapse. Trial pit terminated at 3.4m due to hard digging.					
	<table border="1"> <tr> <td>Scale (approx)</td> <td>Logged By</td> <td>Figure No.</td> </tr> <tr> <td>1:25</td> <td>MS</td> <td>9225-11-19.TP07</td> </tr> </table>	Scale (approx)	Logged By	Figure No.	1:25	MS
Scale (approx)	Logged By	Figure No.				
1:25	MS	9225-11-19.TP07				



Machine : 8.5T Excavator Method : Trial Pit	Dimensions 1.0m x 2.5m x 4.0m	Ground Level (mOD) 23.42	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725105.6 E 759041.7 N	Dates 28/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B1			23.32	(0.10)	TOPSOIL.		
					(0.35)	Soft brown slightly sandy slightly gravelly silty CLAY. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
1.00	B2			22.97	0.45	Soft to firm orange brown slightly sandy slightly gravelly CLAY with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded fine to coarse.		
					(1.05)			
2.00	B3			21.92	1.50	Firm orange brown slightly sandy slightly gravelly CLAY with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded fine to coarse.		
					(1.00)			
3.50	B4		Water strike(1) at 2.50m.	20.92	2.50	Firm to stiff orange brown sandy slightly gravelly CLAY with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded fine to coarse.		∇ ₁
					(0.40)			
				20.52	2.90	Stiff orange brown slightly sandy slightly gravelly CLAY with occasional sub-angular to sub-rounded cobbles and boulders. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
					(1.10)			
				19.42	4.00			

Plan .	Remarks Trial pit backfilled on completion. Minor groundwater seepage from 2.5m to 3.5m. Minor trial pit side wall collapse from 2.5m to 3.5m.	
		Scale (approx) 1:25



Machine : 8.5T Excavator Method : Trial Pit	Dimensions 1.0m x 2.5m x 2.7m	Ground Level (mOD) 21.94	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725089.1 E 759118.7 N	Dates 29/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			21.74	(0.20) 0.20	Brown slightly gravelly slightly sandy TOPSOIL.		
1.50	B		Water strike(1) at 1.80m.		(2.30)	Soft brown sandy gravelly silty CLAY with occasional sub-angular to sub-rounded cobbles. Gravels are sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		∇1
				19.44	2.50 (0.20)	Stiff brown sandy gravelly silty CLAY with occasional sub-angular to sub-rounded cobbles. Gravels are sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
				19.24	2.70	Complete at 2.70m		

Plan .	Remarks Groundwater encountered at 1.8m. High amount of water causing weakness and major sidewall collapse. Trial pit terminated due to instability. Trial pit backfilled on completion.	
		Scale (approx) 1:25



Machine : 8.5T Excavator Method : Trial Pit		Dimensions		Ground Level (mOD) 19.24		Client DBFL		Job Number 9225-11-19	
		Location (dGPS) 725030.4 E 759208.9 N		Dates 29/11/2019		Project Contractor Ground Investigations Ireland		Sheet 1/1	

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			19.14	(0.10) 0.10	TOPSOIL with rootlets. Soft brown sandy gravelly silty CLAY with some angular to sub-angular cobbles. Gravels are sub-angular to sub-rounded fine to coarse. Sand is fine to coarse.		
			Water strike(1) at 1.00m.		(1.70)			∇1
1.50	B		Water strike(2) at 1.50m.					∇2
				17.44	1.80	Firm brown sandy gravelly silty CLAY with some angular to sub-angular cobbles. Gravels are sub-angular to sub-rounded fine to coarse. Sand is fine to coarse.		
					(0.70)			
2.50	B			16.74	2.50	Stiff brown sandy gravelly silty CLAY with some angular to sub-angular cobbles. Gravels are sub-angular to sub-rounded fine to coarse. Sand is fine to coarse.		
					(0.90)			
				15.84	3.40	Complete at 3.40m		

Plan					Remarks				
.					Groundwater seepage encountered at 1.00m and at 1.50m.				
.					Trial pit unstable.				
.					Trial pit terminated due to instability.				
.					Trial pit backfilled on completion.				
.					Scale (approx)		Logged By		Figure No.
.					1:25		MMC		9225-11-19.TP11



Machine : 8.5T Excavator Method : Trial Pit	Dimensions	Ground Level (mOD) 16.16	Client DBFL	Job Number 9225-11-19
	Location 724994.3 E 759248.2 N	Dates 27/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			15.96	(0.20)	Brown slightly sandy slightly gravelly TOPSOIL.		
					0.20	Soft brown sandy gravelly silty CLAY. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
1.00	B			15.36	(0.60)			
					0.80	Loose greyish brown clayey slightly gravelly SAND. Gravel is sub-angular to sub-rounded, fine to coarse.		
				14.66	1.50	Complete at 1.50m		▼1

Water strike(1) at 1.50m, rose to 1.30m in 5 mins.

Plan
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Remarks		
Groundwater encountered at 1.5m. Water filled back to 1.3m BGL after 5min. Trial pit terminated at 1.5m due to groundwater inflow. Trial pit backfilled on completion.		
Scale (approx)	Logged By	Figure No.
1:25	MMC	9225-11-19.TP12



Machine : 8.5T Excavator Method : Trial Pit	Dimensions 1.0m x 2.5m x 2.6m	Ground Level (mOD) 19.43	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725035.4 E 759292.5 N	Dates 29/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			19.28	(0.15) 0.15	MADE GROUND: Angular fine to coarse gravel.		
					(0.65)	MADE GROUND: Brown sandy very gravelly CLAY with many cobbles. Rare fragments of plastic present. Gravel is sub-angular to sub-rounded, fine to coarse.		
1.00	B			18.63	0.80	MADE GROUND: Brown slightly clayey gravelly SAND with some sub-rounded cobbles. Gravel is sub-angular to sub-rounded, fine to coarse.		
					(1.80)			
					2.60	Complete at 2.60m		
				16.83				

Plan .	Remarks Trial pit stable. Trial pit backfilled on completion. No groundwater encountered during excavation.					
	<table border="1"> <tr> <td>Scale (approx)</td> <td>Logged By</td> <td>Figure No.</td> </tr> <tr> <td>1:25</td> <td>MMC</td> <td>9225-11-19.TP13</td> </tr> </table>	Scale (approx)	Logged By	Figure No.	1:25	MMC
Scale (approx)	Logged By	Figure No.				
1:25	MMC	9225-11-19.TP13				



Machine : 8.5T Excavator Method : Trial Pit		Dimensions 2.1m x 0.6m x 2.0m (L x W x D)	Ground Level (mOD) 25.54	Client DBFL	Job Number 9225-11-19
		Location 724983.3 E 759070.1 N	Dates 27/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (Thickness) (m)	Description	Legend	Water
0.50	B			25.39	(0.15)	TOPSOIL.		
					0.15	Soft brown sandy gravelly silty CLAY with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
1.50	B			24.54	(0.85)			
					1.00	Medium dense brown gravelly slightly clayey SAND with occasional sub-angular to sub-rounded cobbles and occasional sub-rounded boulders. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
					(1.00)			
				23.54	2.00	Dense brown gravelly slightly clayey SAND with occasional sub-angular to sub-rounded cobbles and occasional sub-rounded boulders. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
					(0.60)			
				22.94	2.60	Brown slightly clayey gravelly SAND with occasional sub-angular to sub-rounded cobbles and occasional sub-rounded boulders. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
				21.54	4.00			

Plan .	Remarks No groundwater encountered during excavation. Trial pit stable. Soakaway test carried out in pit at 2.00m BGL. Trial pit backfilled on completion.					
	<table border="1"> <tr> <td>Scale (approx)</td> <td>Logged By</td> <td>Figure No.</td> </tr> <tr> <td>1:25</td> <td>MMC</td> <td>9225-11-19.TP114</td> </tr> </table>	Scale (approx)	Logged By	Figure No.	1:25	MMC
Scale (approx)	Logged By	Figure No.				
1:25	MMC	9225-11-19.TP114				



Machine : 8.5T Excavator Method : Trial Pit		Dimensions 2.5m x 0.5m x 2.0m (L x W x D)	Ground Level (mOD) 23.15	Client DBFL	Job Number 9225-11-19
		Location 725099.2 E 759083.8 N	Dates 27/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			23.00	(0.15)	TOPSOIL.		
					0.15	Soft brown sandy gravelly/silty CLAY with occasional angular to sub-angular cobbles. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
1.50	B				(1.65)			
2.50	B			21.35	1.80	Firm to stiff brown sandy gravelly/silty CLAY with occasional angular to sub-angular cobbles. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
				21.05	2.10	Stiff brown sandy gravelly silty CLAY with occasional angular to sub-angular cobbles. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
				20.15	3.00	Complete at 3.00m		

Plan .	Remarks No groundwater encountered during excavation. Trial pit stable. Soakaway test carried out in pit at 2.0mBGL Trial pit terminated due to hard digging at 3m.					
	<table border="1"> <tr> <td>Scale (approx)</td> <td>Logged By</td> <td>Figure No.</td> </tr> <tr> <td>1:25</td> <td>MMC</td> <td>9225-11-19.TPI15</td> </tr> </table>	Scale (approx)	Logged By	Figure No.	1:25	MMC
Scale (approx)	Logged By	Figure No.				
1:25	MMC	9225-11-19.TPI15				



Machine : 8.5T Excavator Method : Trial Pit		Dimensions 2.2m x 0.6m x 2.00m (L x W x D)	Ground Level (mOD) 24.06	Client DBFL	Job Number 9225-11-19
		Location (dGPS) 724961.1 E 759157.1 N	Dates 27/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			23.91	(0.15)	TOPSOIL.		
					0.15	Soft brown sandy gravelly silty CLAY with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
1.50	B			23.06	(0.85)			
					1.00	Brown slightly clayey gravelly SAND with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		
2.50	B			21.96	(1.10)			
					2.10	Brown very gravelly slightly clayey SAND with many sub-rounded cobbles.		
3.50	B			20.06	(1.90)			
					4.00			
			Water strike(1) at 3.70m.					

Plan .	Remarks Moderate groundwater encountered at 3.7m, moderate flow. Trial Pit Stable. Soakaway test carried out in pit at 2.0m Trial Pit backfilled on completion.		
	Scale (approx) 1:25	Logged By MMC	Figure No. 9225-11-19.TPI16



Machine : 8.5T Excavator Method : Trial Pit	Dimensions	Ground Level (mOD) 15.13	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725145 E 759187.4 N	Dates 27/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			14.98	(0.15)	TOPSOIL		
					0.15	MADE GROUND: Brown sandy slightly gravelly silty clay. Rare fragments of bricks/clay pipe.		
1.50	B		Water strike(1) at 1.40m.	14.43	(0.55)	Soft to firm grey mottled orange sandy slightly gravelly silty CLAY. Gravel is sub-angular to sub-rounded, fine to coarse.		∇1
					0.70			
2.00	B			13.43	(1.00)	Brownish grey clayey sandy GRAVEL. Gravel is sub-angular to sub-rounded.		
					1.70			
				13.03	2.10	Complete at 2.10m		

Plan .	Remarks Trial Pit terminated due to collapse in gravel with presence of water. Pit filling with water - unsuitable for soakaway. Groundwater encountered at 1.40m. Fast flow.	
		Scale (approx) 1:25



Machine : 8.5T Excavator Method : Trial Pit	Dimensions 2.4m x 0.5m x 2.00m (L x W x D)	Ground Level (mOD) 16.10	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724942.5 E 759265.5 N	Dates 27/11/2019	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.50	B			15.90	(0.20) 0.20	TOPSOIL.		
				15.40	(0.50) 0.70	Soft brown sandy gravelly CLAY. Gravel is sub-angular to sub-rounded, fine to coarse.		
1.50	B		Water strike(1) at 1.00m.		(1.30) 2.00	Soft to firm grey matt brown sandy silty CLAY with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.		∇1
				14.10	2.00	Complete at 2.00m		

Plan .	Remarks Groundwater seepage encountered at 1.00m. Trial pit stable. Soakway test carried out in pit at 2.0 mBGL Trial pit backfilled on completion.		
	<table border="1"> <tr> <td>Scale (approx) 1:25</td> <td>Logged By MMC</td> <td>Figure No. 9225-11-19.TPI18</td> </tr> </table>	Scale (approx) 1:25	Logged By MMC
Scale (approx) 1:25	Logged By MMC	Figure No. 9225-11-19.TPI18	

**Hackettstown, Skerries, Co. Dublin –
Trial Pit Photos**

TP01



TP01



TP01



TP01



TP02



TP02



TP02



TP02



TP02



TP03



TP03



TP03



TP03



TP03



TP03



TP04



TP04



TP04



TP04



TP05



TP05



TP05



TP05



TP05



TP06



TP06



TP06



TP06



TP07



TP07



TP07



TP07



TP07



TP08



TP08



TP08



TP08



TP08



TP09



TP09



TP09



TP09



TP09



TP10



TP10



TP10



TP10



TP11



TP11



TP11



TP11



TP12



TP12



TP12



TP12



TP13



TP13



TP13



TP13



TPI14



TPI14



TPI14



TPI14



TPI15



TPI15



TPI15



TPI15



TPI15



TPI16



TPI16



TPI16



TPI16



TPI17



TPI17



TPI17



TPI17



TPI17



TPI17A



TPI17A



TPI17A



TPI17A



TPI18



TPI18



TPI18



TPI18



APPENDIX 3 – Soakaway Results





GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

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TPI14

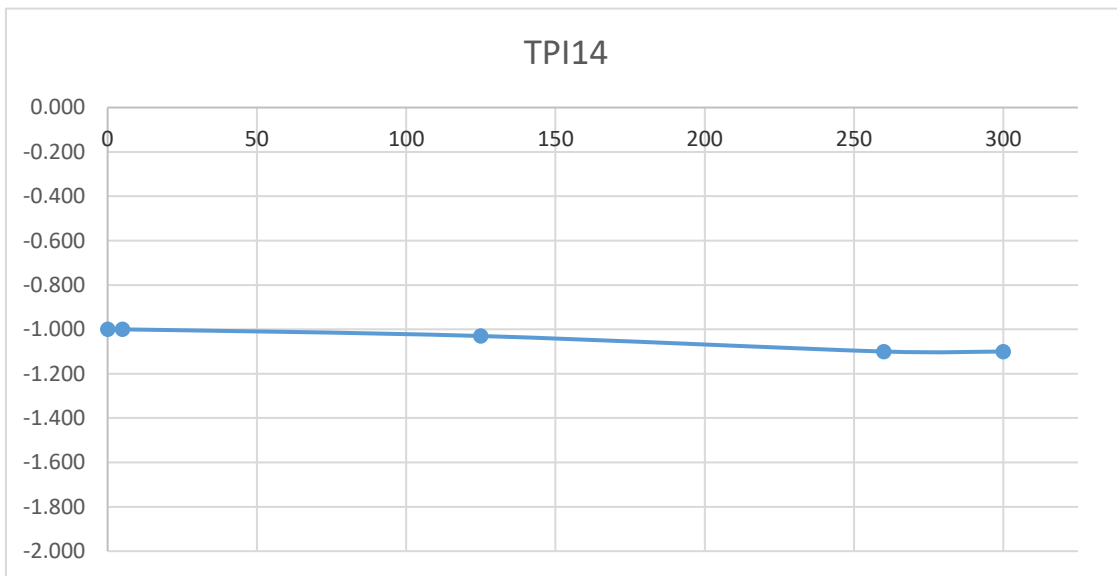
Soakaway Test to BRE Digest 365

Trial Pit Dimensions: 2.10m X 0.60m X 2.00m (L x W x D)

Date	Time	Water level (m bgl)
27/11/2019	0	-1.000
27/11/2019	5	-1.000
27/11/2019	125	-1.030
27/11/2019	260	-1.100
27/11/2019	300	-1.100

***Soakaway failed - Pit backfilled**

Start depth	Depth of Pit	Diff	75% full	25%full
1.00	2.000	1.000	1.25	1.75





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Web: www.gii.ie

TPI15

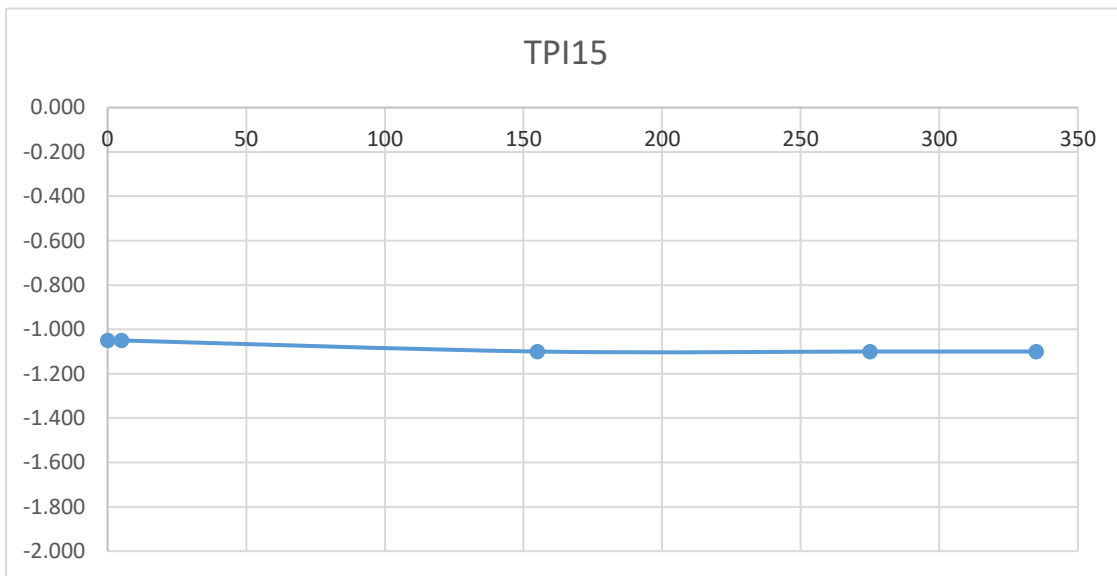
Soakaway Test to BRE Digest 365

Trial Pit Dimensions: 2.50m X 0.50m X 2.00m (L x W x D)

Date	Time	Water level (m bgl)
27/11/2019	0	-1.050
27/11/2019	5	-1.050
27/11/2019	155	-1.100
27/11/2019	275	-1.100
27/11/2019	335	-1.100

***Soakaway failed - Pit backfilled**

Start depth	Depth of Pit	Diff	75% full	25%full
1.05	2.000	0.950	1.2875	1.7625





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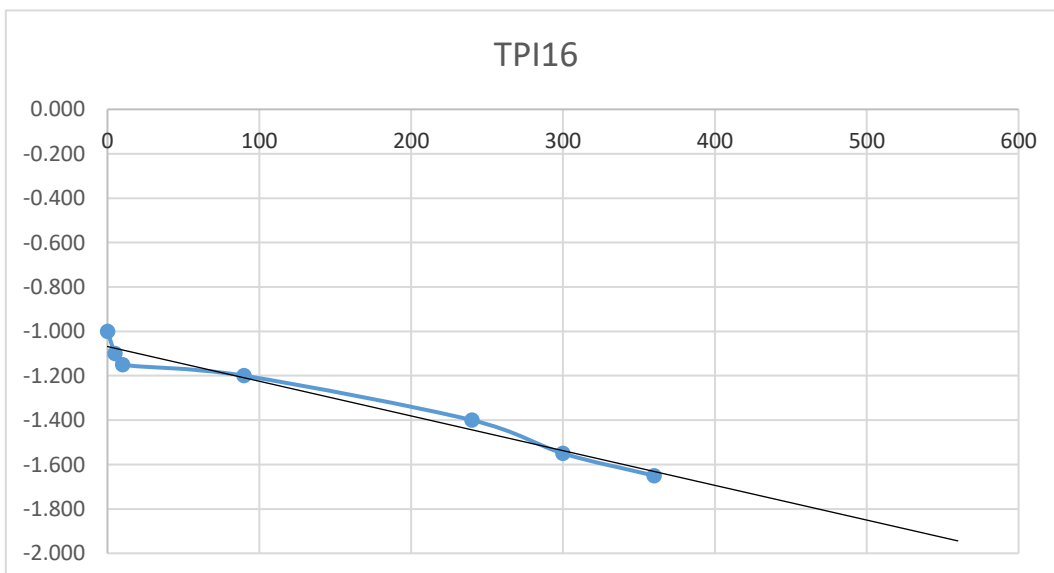
TPI16

Soakaway Test to BRE Digest 365

Trial Pit Dimensions: 2.20m X 0.60m X 2.00m (L x W x D)

Date	Time	Water level (m bgl)
27/11/2019	0	-1.000
27/11/2019	5	-1.100
27/11/2019	10	-1.150
27/11/2019	90	-1.200
27/11/2019	240	-1.400
27/11/2019	300	-1.550
27/11/2019	360	-1.650

Start depth 1.00	Depth of Pit 2.000	Diff 1.000	75% full 1.25	25%full 1.75
Length of pit (m)	Width of pit (m)		75-25Ht (m)	Vp75-25 (m3)
2.200	0.600		0.500	0.66
Tp75-25 (from graph) (s)	19500		50% Eff Depth	ap50 (m2)
			0.500	4.12
f =	8.215E-06	m/s		





GROUND INVESTIGATIONS IRELAND
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D22 YD52

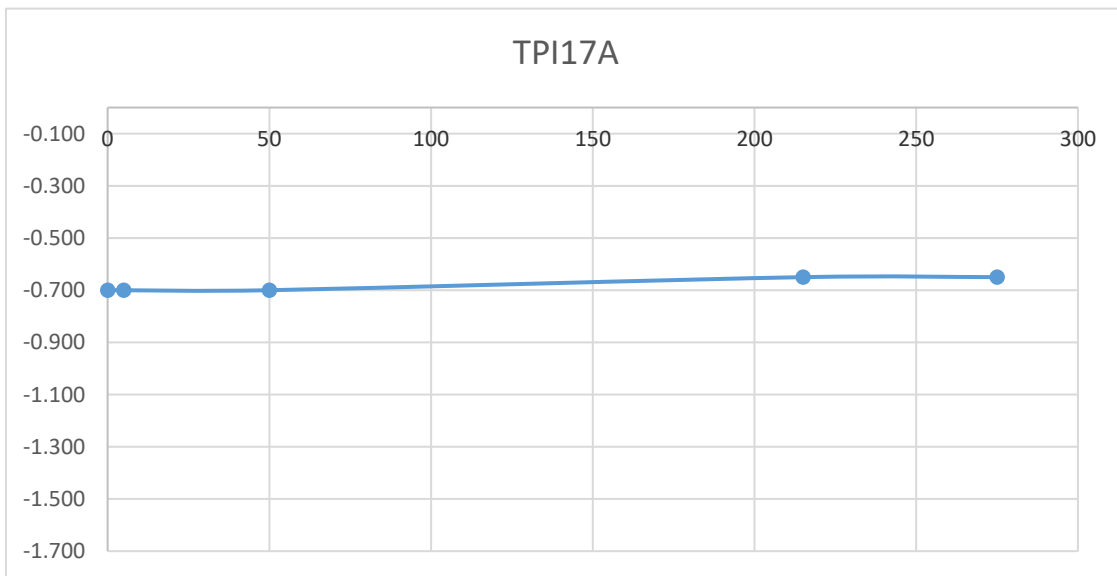
Tel: 01 601 5175 / 5176
Email: info@gii.ie
Web: www.gii.ie

TPI17A
Soakaway Test to BRE Digest 365
Trial Pit Dimensions: 2.20m X 0.5m X 1.70m (L x W x D)

Date	Time	Water level (m bgl)
27/11/2019	0	-0.700
27/11/2019	5	-0.700
27/11/2019	50	-0.700
27/11/2019	215	-0.650
27/11/2019	275	-0.650

***Soakaway failed - Pit backfilled**

Start depth	Depth of Pit	Diff	75% full	25%full
0.70	1.700	1.000	0.95	1.45





GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

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Web: www.gii.ie

TPI18

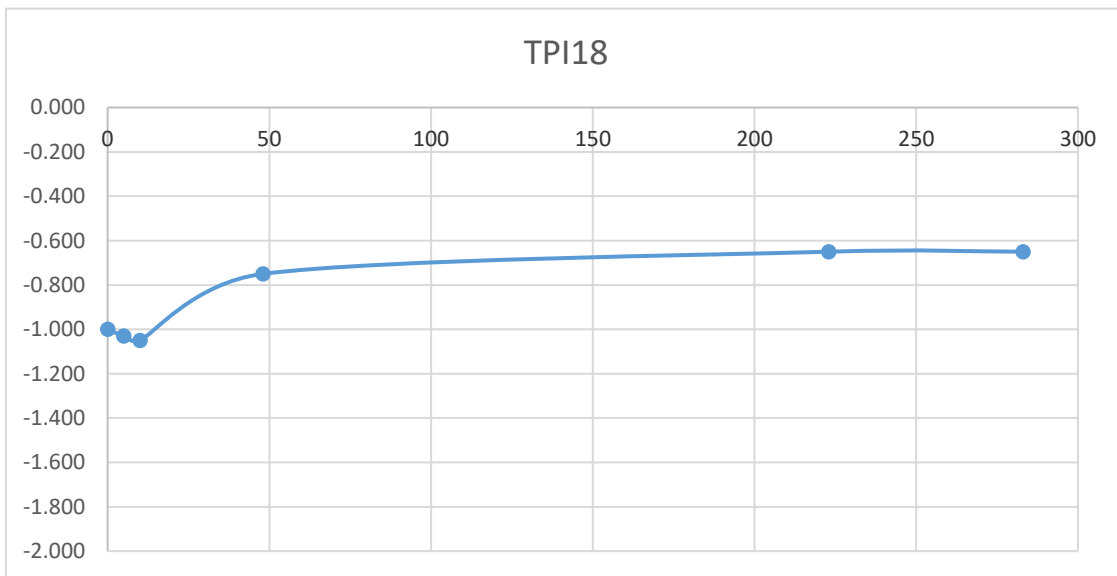
Soakaway Test to BRE Digest 365

Trial Pit Dimensions: 2.40m X 0.50m X 2.00m (L x W x D)

Date	Time	Water level (m bgl)
27/11/2019	0	-1.000
27/11/2019	5	-1.030
27/11/2019	10	-1.050
27/11/2019	48	-0.750
27/11/2019	223	-0.650
27/11/2019	283	-0.650

***Soakaway failed - Pit backfilled**

Start depth	Depth of Pit	Diff	75% full	25%full
1.00	2.000	1.000	1.25	1.75



APPENDIX 4 – Dynamic Probe Records





Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 18.96	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724926.1 E 759225.3 N	Dates 21/02/2020	Engineer DBFL	Sheet 1/2

Depth (m)	Blows for Depth Increment	Field Records	Level (mOD)	Depth (m)	Blows for Depth Increment												
					0	3	6	9	12	15	18	21	24	27	30		
0.00-0.10	4		18.96	0.00													
0.10-0.20	3																
0.20-0.30	4																
0.30-0.40	3																
0.40-0.50	3																
0.50-0.60	3		18.46	0.50													
0.60-0.70	2																
0.70-0.80	2																
0.80-0.90	2																
0.90-1.00	3																
1.00-1.10	0		17.96	1.00													
1.10-1.20	0																
1.20-1.30	4																
1.30-1.40	4																
1.40-1.50	6																
1.50-1.60	6		17.46	1.50													
1.60-1.70	8																
1.70-1.80	8																
1.80-1.90	4																
1.90-2.00	4																
2.00-2.10	4		16.96	2.00													
2.10-2.20	4																
2.20-2.30	4																
2.30-2.40	4																
2.40-2.50	5																
2.50-2.60	5		16.46	2.50													
2.60-2.70	4																
2.70-2.80	5																
2.80-2.90	5																
2.90-3.00	7																
3.00-3.10	7		15.96	3.00													
3.10-3.20	7																
3.20-3.30	8																
3.30-3.40	6																
3.40-3.50	5																
3.50-3.60	10		15.46	3.50													
3.60-3.70	11																
3.70-3.80	13																
3.80-3.90	13																
3.90-4.00	10																
4.00-4.10	13		14.96	4.00													
4.10-4.20	13																
4.20-4.30	15																
4.30-4.40	12																
4.40-4.50	11																
4.50-4.60	14		14.46	4.50													
4.60-4.70	15																
4.70-4.80	19																
4.80-4.90	20																
4.90-5.00	21		13.96	5.00													

Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP01



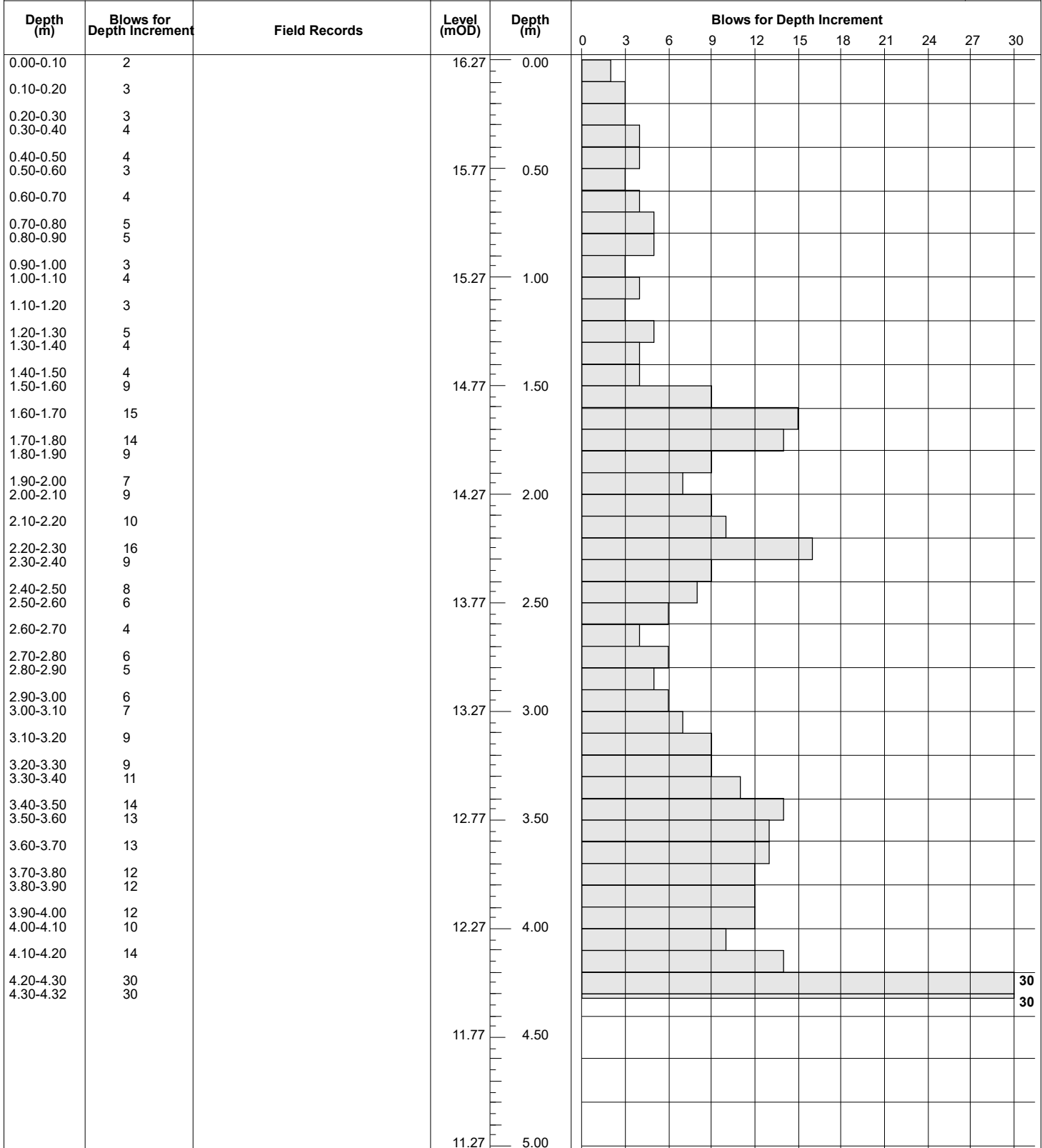
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 18.96	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724926.1 E 759225.3 N	Dates 21/02/2020	Engineer DBFL	Sheet 2/2

Depth (m)	Blows for Depth Increment	Field Records	Level (mOD)	Depth (m)	Blows for Depth Increment																	
					0	3	6	9	12	15	18	21	24	27	30							
5.00-5.10	24		13.96	5.00																		
			13.46	5.50																		
			12.96	6.00																		
			12.46	6.50																		
			11.96	7.00																		
			11.46	7.50																		
			10.96	8.00																		
			10.46	8.50																		
			9.96	9.00																		
			9.46	9.50																		
			8.96	10.00																		

Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP01



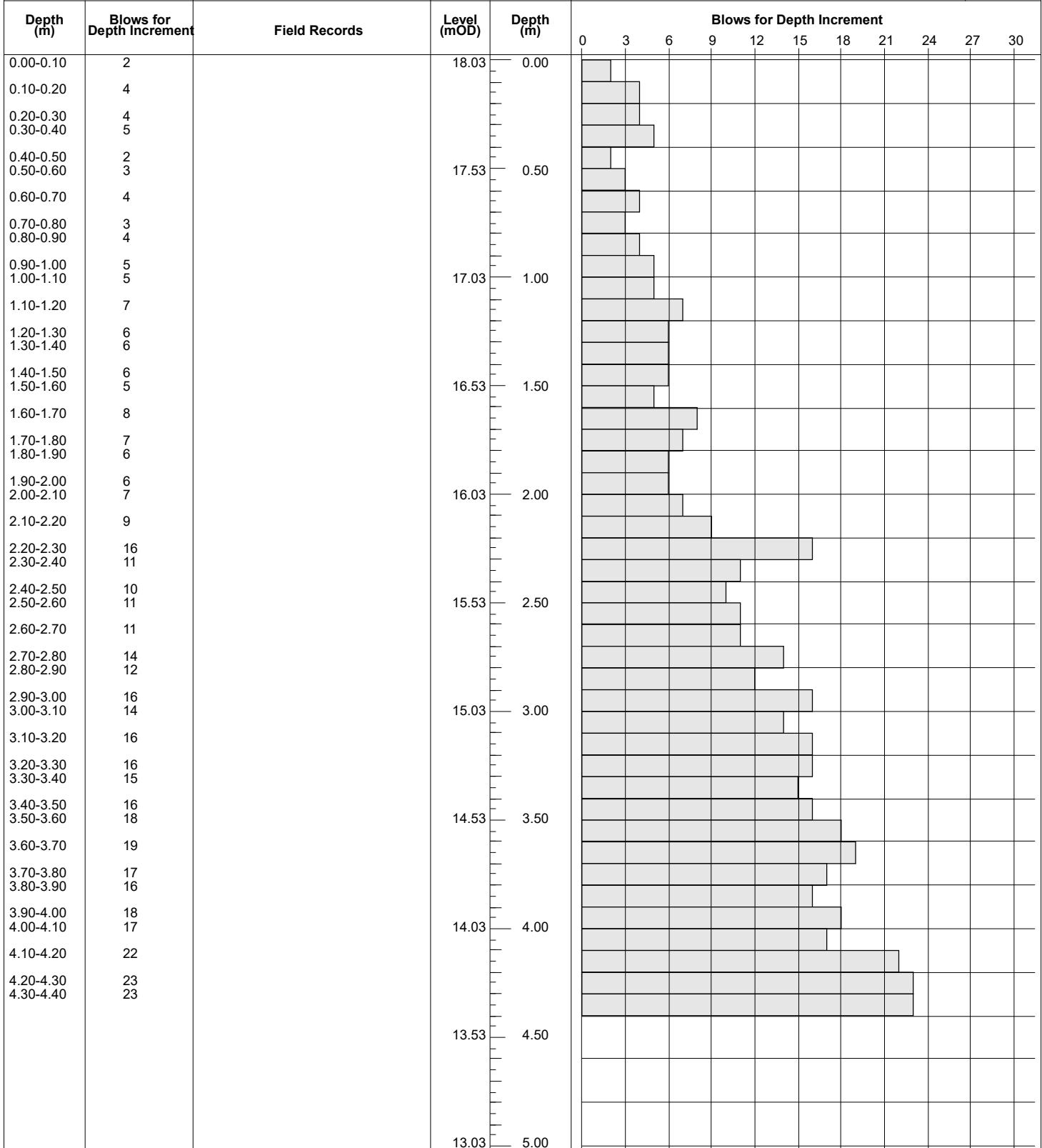
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 16.27	Client DBFL	Job Number 9225-11-19
	Location 724992.6 E 759246.6 N	Dates 21/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP02



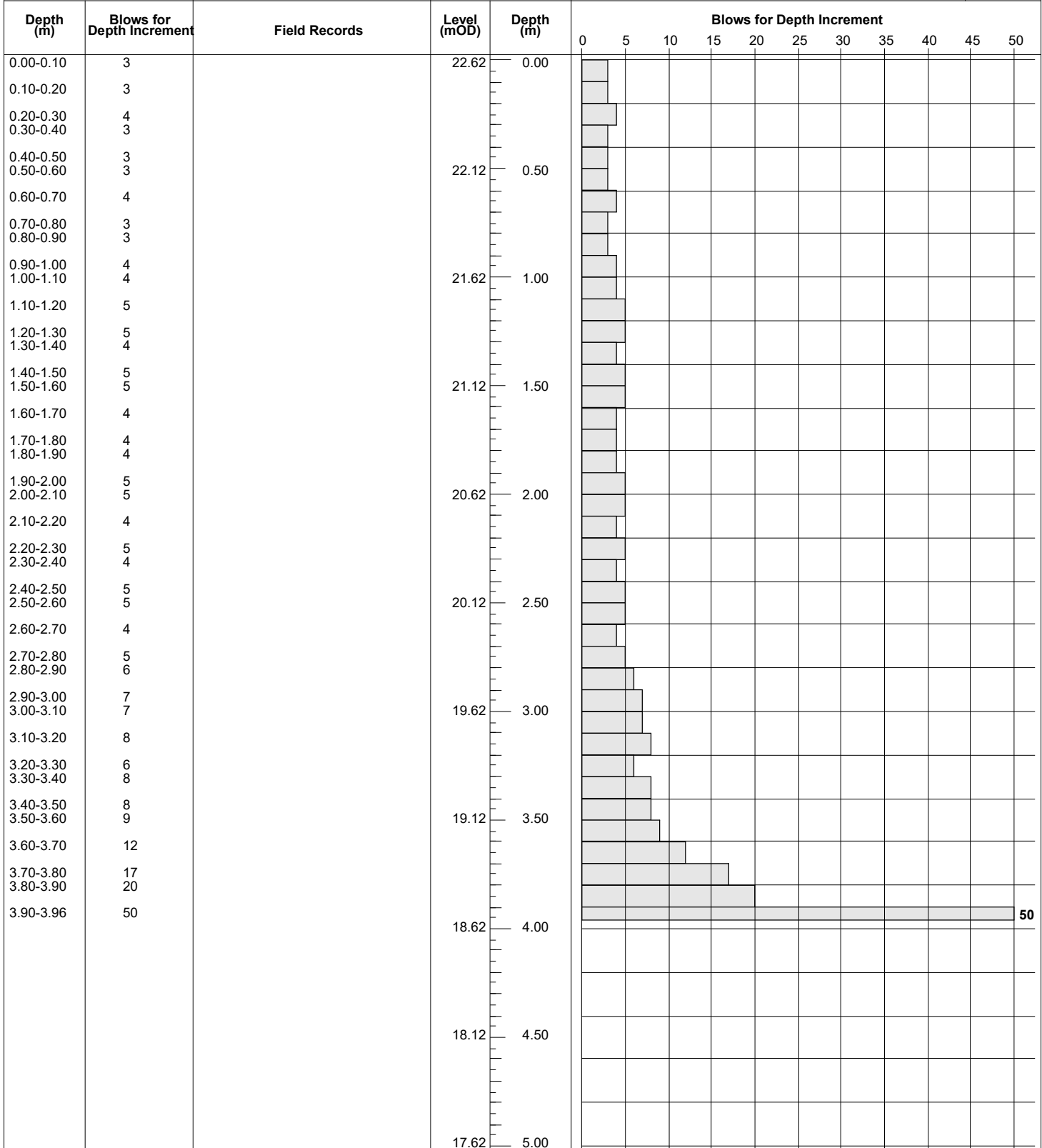
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 18.03	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725083.9 E 759186.9 N	Dates 21/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP03



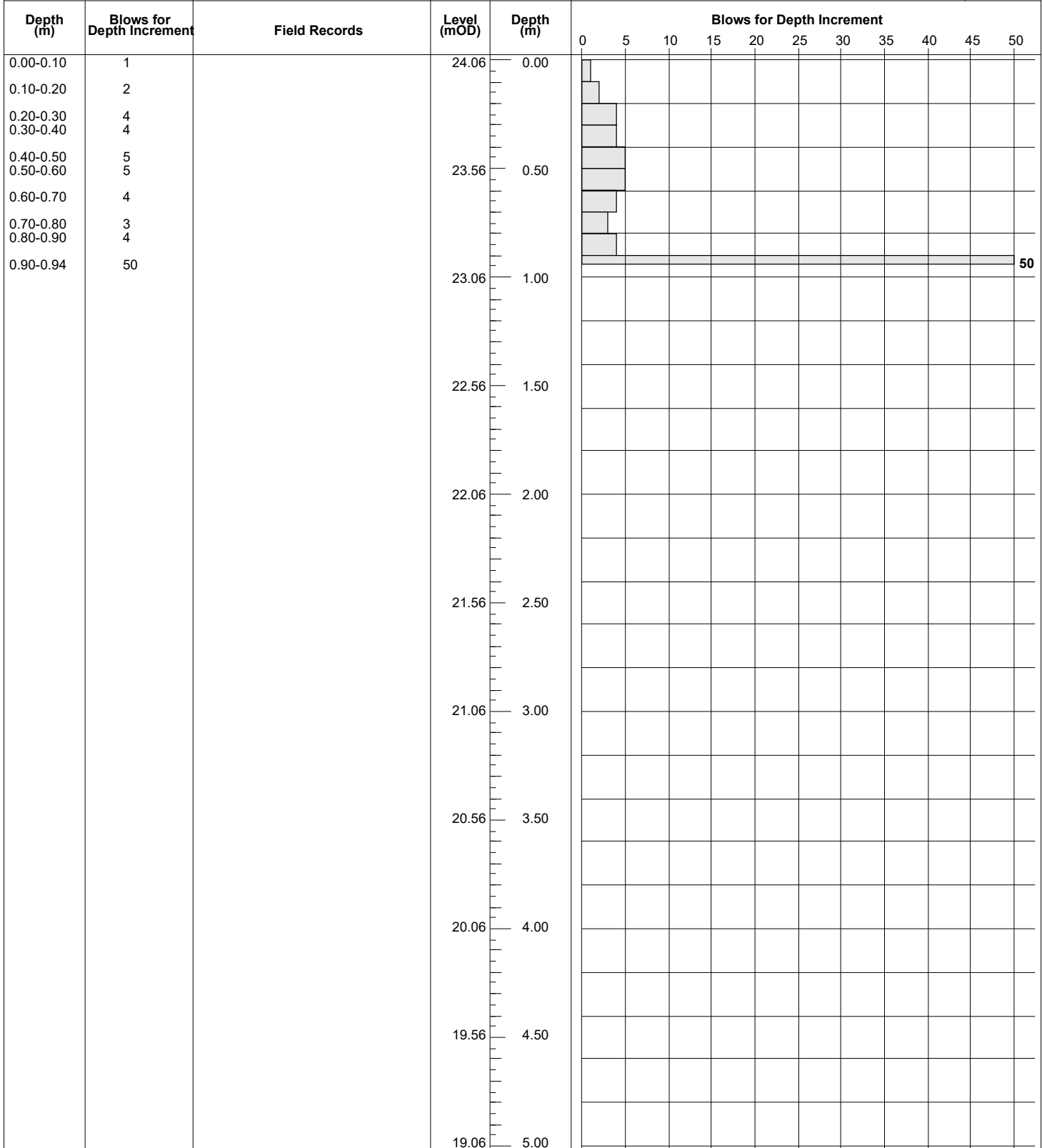
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 22.62	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724917.1 E 759165.5 N	Dates 22/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP04



Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 24.06	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724968.5 E 759167.1 N	Dates 22/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP05



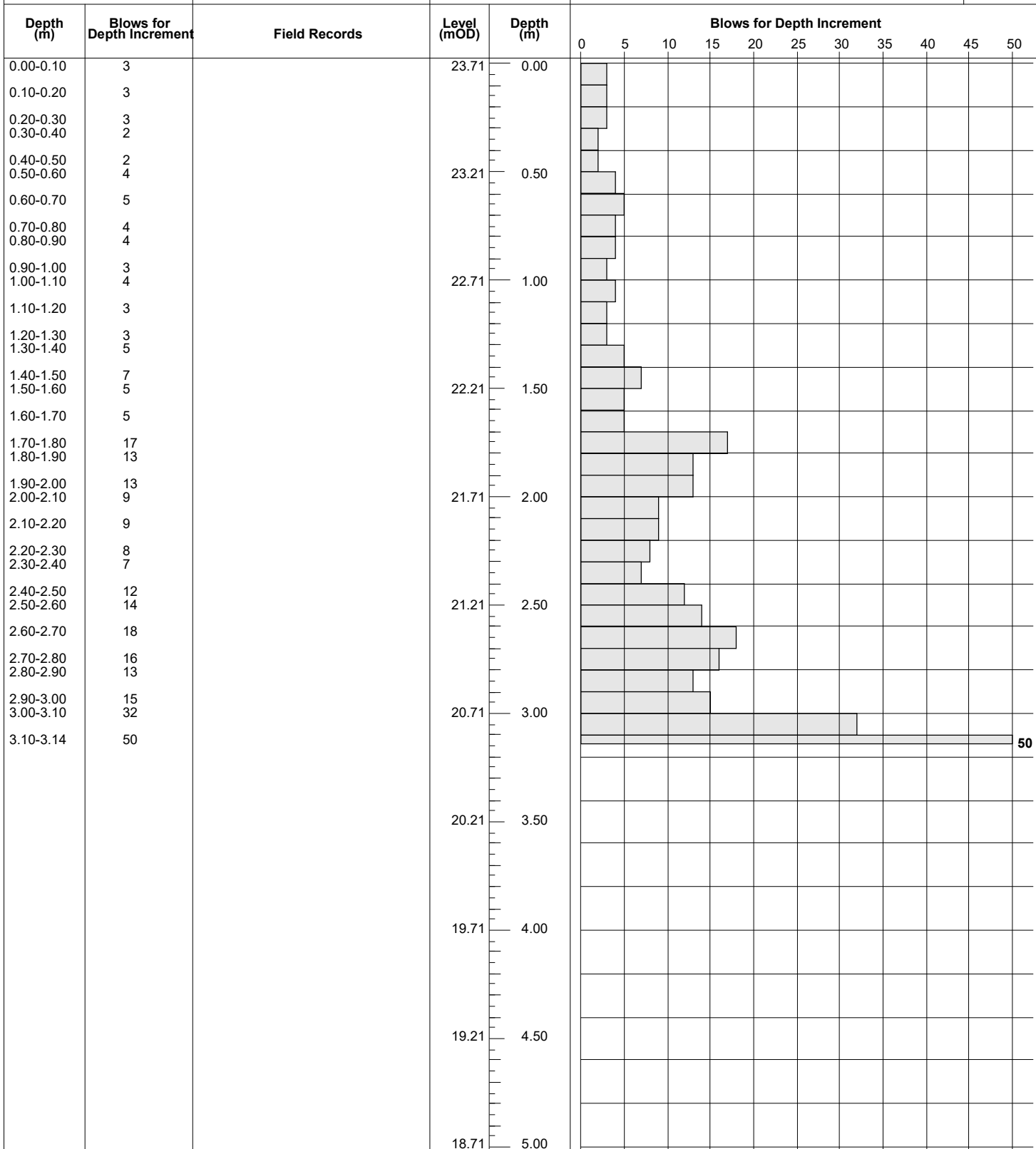
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 24.06	Client DBFL	Job Number 9225-11-19
	Location 724968.5 E 759167.1 N	Dates 22/02/2020- 22/03/2020	Engineer DBFL	Sheet 1/1

Depth (m)	Blows for Depth Increment	Field Records	Level (mOD)	Depth (m)	Blows for Depth Increment										
					0	4	8	12	16	20	24	28	32	36	40
0.00-0.10	2		24.06	0.00	[Bar chart showing 2 blows]										
0.10-0.20	2				[Bar chart showing 2 blows]										
0.20-0.30	4				[Bar chart showing 4 blows]										
0.30-0.40	3				[Bar chart showing 3 blows]										
0.40-0.50	4				[Bar chart showing 4 blows]										
0.50-0.60	3		23.56	0.50	[Bar chart showing 3 blows]										
0.60-0.70	3				[Bar chart showing 3 blows]										
0.70-0.80	3				[Bar chart showing 3 blows]										
0.80-0.90	12				[Bar chart showing 12 blows]										
0.90-1.00	16				[Bar chart showing 16 blows]										
1.00-1.10	27		23.06	1.00	[Bar chart showing 27 blows]										
1.10-1.20	26				[Bar chart showing 26 blows]										
1.20-1.30	23				[Bar chart showing 23 blows]										
1.30-1.40	32				[Bar chart showing 32 blows]										
			22.56	1.50	[Bar chart showing 0 blows]										
			22.06	2.00	[Bar chart showing 0 blows]										
			21.56	2.50	[Bar chart showing 0 blows]										
			21.06	3.00	[Bar chart showing 0 blows]										
			20.56	3.50	[Bar chart showing 0 blows]										
			20.06	4.00	[Bar chart showing 0 blows]										
			19.56	4.50	[Bar chart showing 0 blows]										
			19.06	5.00	[Bar chart showing 0 blows]										

Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP05A



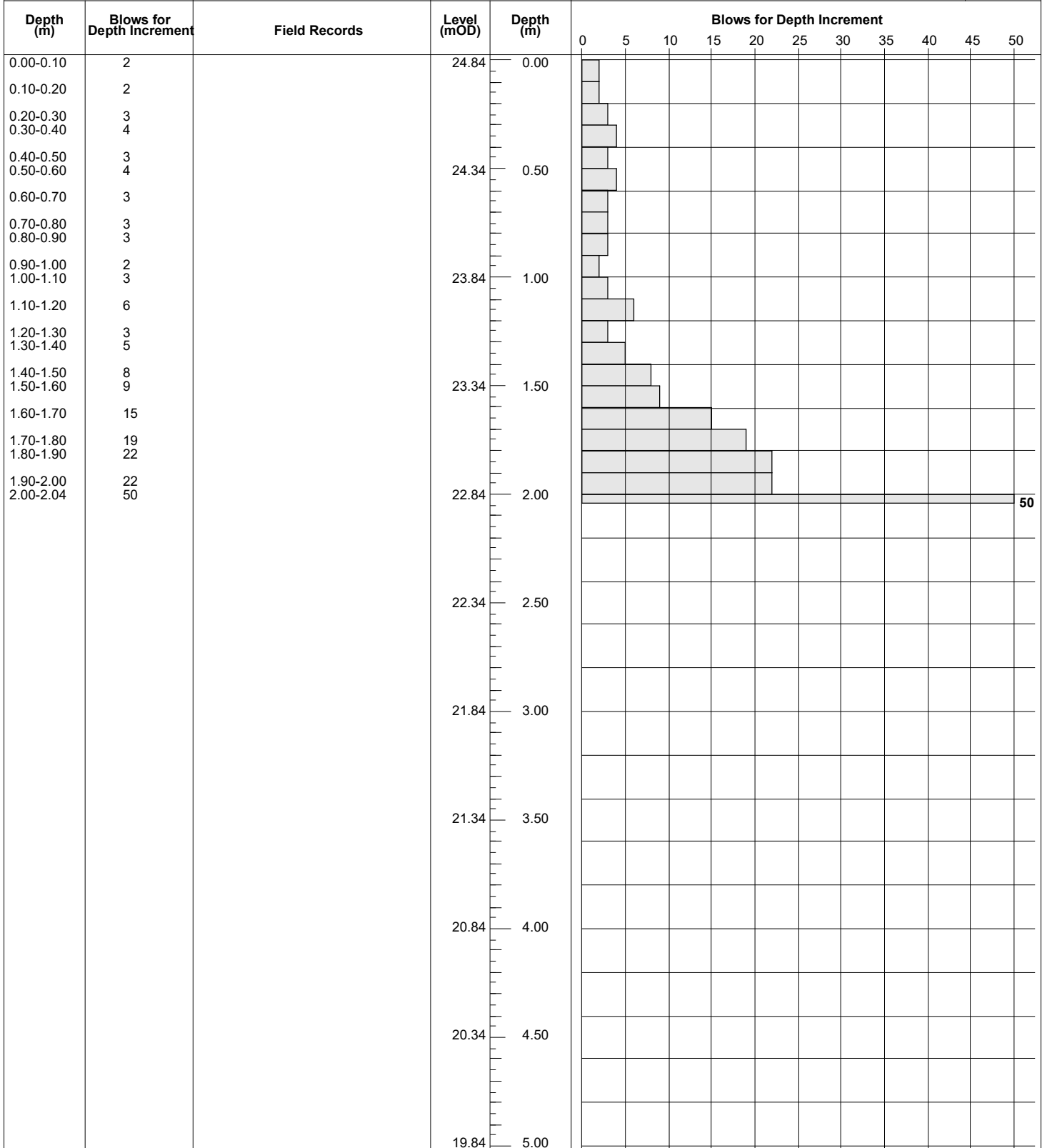
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 23.71	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725020.9 E 759135.6 N	Dates 22/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP06



Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 24.84	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724974.9 E 759121.3 N	Dates 22/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP07



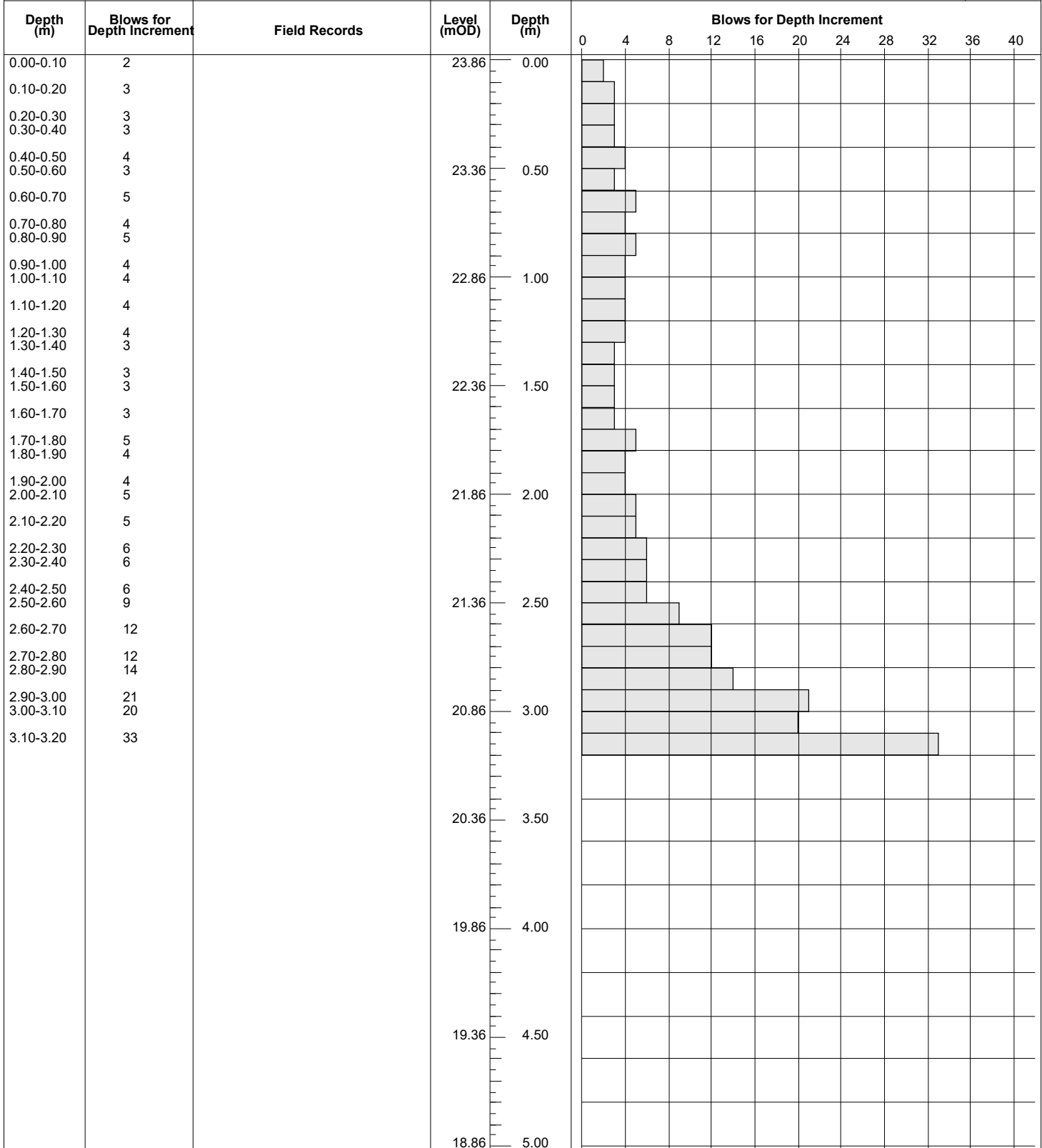
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 23.78	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724913.1 E 759112.2 N	Dates 22/02/2020	Engineer DBFL	Sheet 1/1

Depth (m)	Blows for Depth Increment	Field Records	Level (mOD)	Depth (m)	Blows for Depth Increment										
					0	4	8	12	16	20	24	28	32	36	40
0.00-0.10	3		23.78	0.00	[Bar chart showing 3 blows]										
0.10-0.20	3				[Bar chart showing 3 blows]										
0.20-0.30	2				[Bar chart showing 2 blows]										
0.30-0.40	3				[Bar chart showing 3 blows]										
0.40-0.50	2				[Bar chart showing 2 blows]										
0.50-0.60	3		23.28	0.50	[Bar chart showing 3 blows]										
0.60-0.70	4				[Bar chart showing 4 blows]										
0.70-0.80	4				[Bar chart showing 4 blows]										
0.80-0.90	4				[Bar chart showing 4 blows]										
0.90-1.00	4				[Bar chart showing 4 blows]										
1.00-1.10	3		22.78	1.00	[Bar chart showing 3 blows]										
1.10-1.20	3				[Bar chart showing 3 blows]										
1.20-1.30	3				[Bar chart showing 3 blows]										
1.30-1.40	2				[Bar chart showing 2 blows]										
1.40-1.50	3				[Bar chart showing 3 blows]										
1.50-1.60	2		22.28	1.50	[Bar chart showing 2 blows]										
1.60-1.70	3				[Bar chart showing 3 blows]										
1.70-1.80	3				[Bar chart showing 3 blows]										
1.80-1.90	3				[Bar chart showing 3 blows]										
1.90-2.00	0				[Bar chart showing 0 blows]										
2.00-2.10	3		21.78	2.00	[Bar chart showing 3 blows]										
2.10-2.20	2				[Bar chart showing 2 blows]										
2.20-2.30	3				[Bar chart showing 3 blows]										
2.30-2.40	3				[Bar chart showing 3 blows]										
2.40-2.50	3				[Bar chart showing 3 blows]										
2.50-2.60	2		21.28	2.50	[Bar chart showing 2 blows]										
2.60-2.70	3				[Bar chart showing 3 blows]										
2.70-2.80	3				[Bar chart showing 3 blows]										
2.80-2.90	3				[Bar chart showing 3 blows]										
2.90-3.00	2				[Bar chart showing 2 blows]										
3.00-3.10	4		20.78	3.00	[Bar chart showing 4 blows]										
3.10-3.20	5				[Bar chart showing 5 blows]										
3.20-3.30	4				[Bar chart showing 4 blows]										
3.30-3.40	3				[Bar chart showing 3 blows]										
3.40-3.50	5				[Bar chart showing 5 blows]										
3.50-3.60	5		20.28	3.50	[Bar chart showing 5 blows]										
3.60-3.70	7				[Bar chart showing 7 blows]										
3.70-3.80	5				[Bar chart showing 5 blows]										
3.80-3.90	6				[Bar chart showing 6 blows]										
3.90-4.00	7				[Bar chart showing 7 blows]										
4.00-4.10	7		19.78	4.00	[Bar chart showing 7 blows]										
4.10-4.20	8				[Bar chart showing 8 blows]										
4.20-4.30	10				[Bar chart showing 10 blows]										
4.30-4.40	9				[Bar chart showing 9 blows]										
4.40-4.50	11				[Bar chart showing 11 blows]										
4.50-4.60	28		19.28	4.50	[Bar chart showing 28 blows]										
4.60-4.70	30				[Bar chart showing 30 blows]										
4.70-4.80	31				[Bar chart showing 31 blows]										
			18.78	5.00	[Bar chart showing 31 blows]										

Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP08



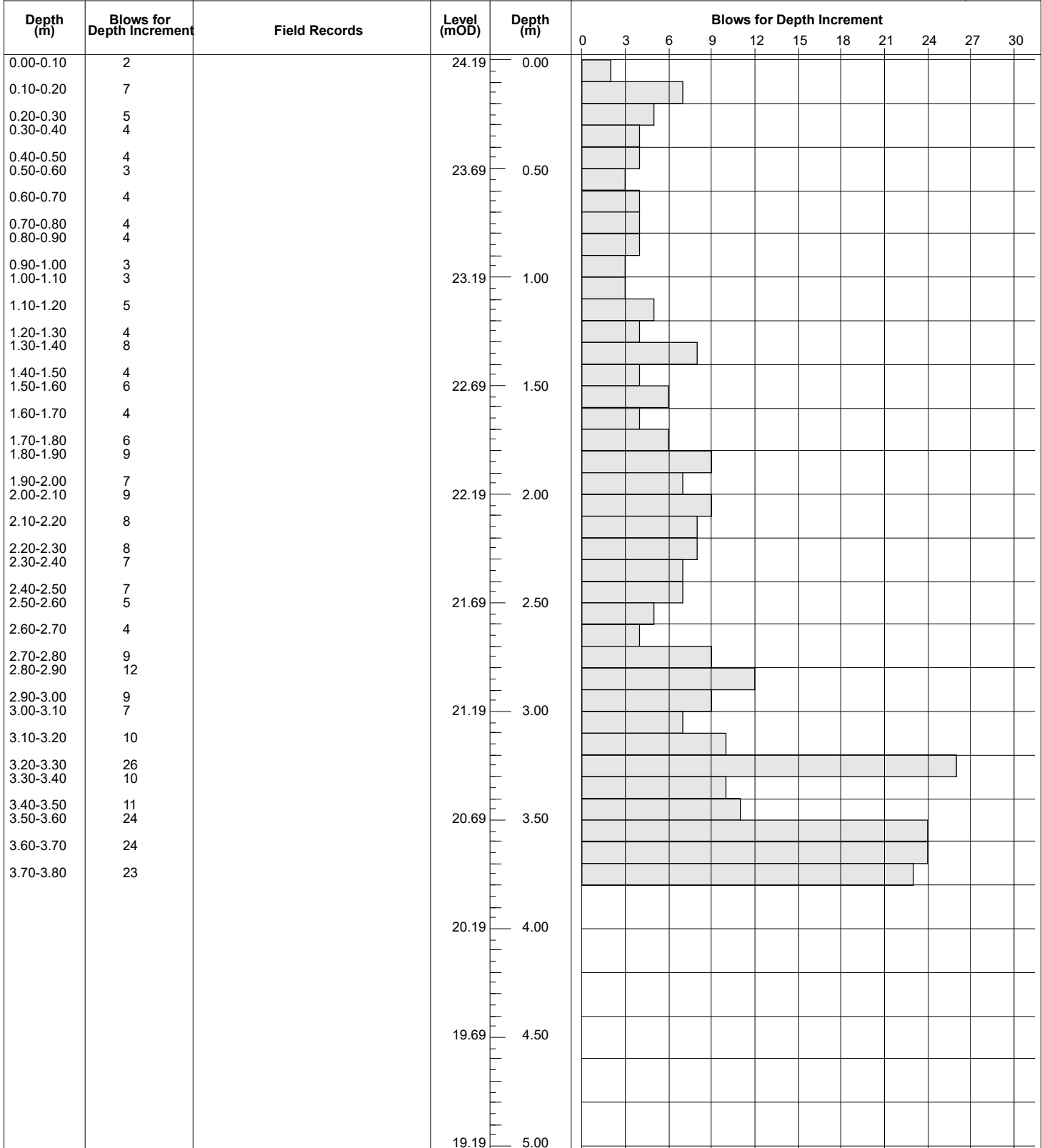
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 23.86	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724924.3 E 759045.5 N	Dates 22/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP09



Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 24.19	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724934.8 E 759012.3 N	Dates 22/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP10



Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 24.85	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724995.3 E 758992.6 N	Dates 22/02/2020	Engineer DBFL	Sheet 1/2

Depth (m)	Blows for Depth Increment	Field Records	Level (mOD)	Depth (m)	Blows for Depth Increment										
					0	5	10	15	20	25	30	35	40	45	50
0.00-0.10	5		24.85	0.00	[Bar chart showing 5 blows]										
0.10-0.20	3				[Bar chart showing 3 blows]										
0.20-0.30	4				[Bar chart showing 4 blows]										
0.30-0.40	3				[Bar chart showing 3 blows]										
0.40-0.50	3				[Bar chart showing 3 blows]										
0.50-0.60	5		24.35	0.50	[Bar chart showing 5 blows]										
0.60-0.70	5				[Bar chart showing 5 blows]										
0.70-0.80	4				[Bar chart showing 4 blows]										
0.80-0.90	6				[Bar chart showing 6 blows]										
0.90-1.00	4				[Bar chart showing 4 blows]										
1.00-1.10	4		23.85	1.00	[Bar chart showing 4 blows]										
1.10-1.20	4				[Bar chart showing 4 blows]										
1.20-1.30	3				[Bar chart showing 3 blows]										
1.30-1.40	4				[Bar chart showing 4 blows]										
1.40-1.50	3				[Bar chart showing 3 blows]										
1.50-1.60	4		23.35	1.50	[Bar chart showing 4 blows]										
1.60-1.70	5				[Bar chart showing 5 blows]										
1.70-1.80	4				[Bar chart showing 4 blows]										
1.80-1.90	5				[Bar chart showing 5 blows]										
1.90-2.00	5				[Bar chart showing 5 blows]										
2.00-2.10	5		22.85	2.00	[Bar chart showing 5 blows]										
2.10-2.20	4				[Bar chart showing 4 blows]										
2.20-2.30	7				[Bar chart showing 7 blows]										
2.30-2.40	10				[Bar chart showing 10 blows]										
2.40-2.50	6				[Bar chart showing 6 blows]										
2.50-2.60	8		22.35	2.50	[Bar chart showing 8 blows]										
2.60-2.70	13				[Bar chart showing 13 blows]										
2.70-2.80	11				[Bar chart showing 11 blows]										
2.80-2.90	12				[Bar chart showing 12 blows]										
2.90-3.00	14				[Bar chart showing 14 blows]										
3.00-3.10	12		21.85	3.00	[Bar chart showing 12 blows]										
3.10-3.20	7				[Bar chart showing 7 blows]										
3.20-3.30	9				[Bar chart showing 9 blows]										
3.30-3.40	17				[Bar chart showing 17 blows]										
3.40-3.50	17				[Bar chart showing 17 blows]										
3.50-3.60	11		21.35	3.50	[Bar chart showing 11 blows]										
3.60-3.70	8				[Bar chart showing 8 blows]										
3.70-3.80	6				[Bar chart showing 6 blows]										
3.80-3.90	8				[Bar chart showing 8 blows]										
3.90-4.00	8				[Bar chart showing 8 blows]										
4.00-4.10	8		20.85	4.00	[Bar chart showing 8 blows]										
4.10-4.20	11				[Bar chart showing 11 blows]										
4.20-4.30	10				[Bar chart showing 10 blows]										
4.30-4.40	11				[Bar chart showing 11 blows]										
4.40-4.50	10				[Bar chart showing 10 blows]										
4.50-4.60	14		20.35	4.50	[Bar chart showing 14 blows]										
4.60-4.70	11				[Bar chart showing 11 blows]										
4.70-4.80	9				[Bar chart showing 9 blows]										
4.80-4.90	9				[Bar chart showing 9 blows]										
4.90-5.00	9		19.85	5.00	[Bar chart showing 9 blows]										

Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP11



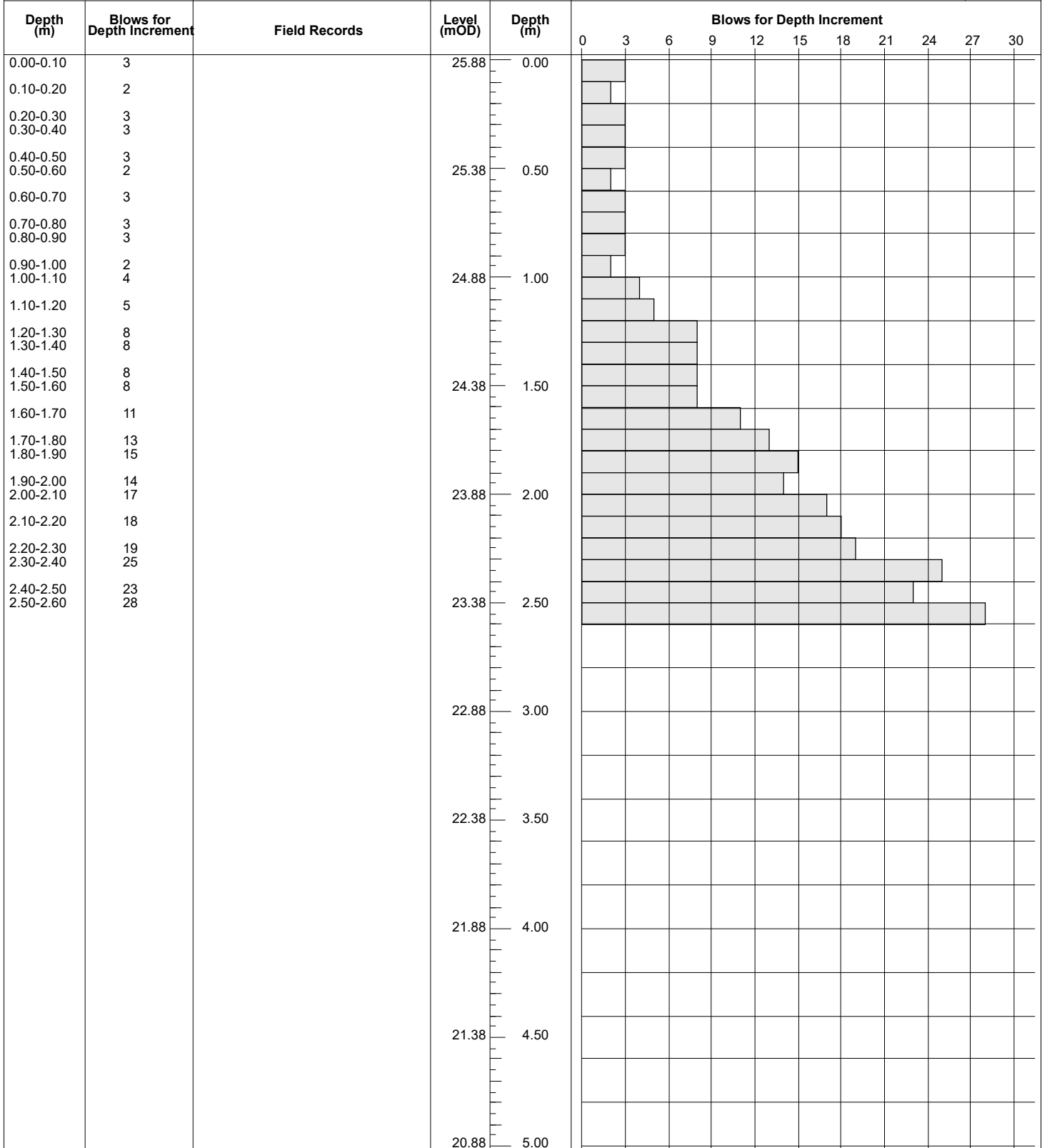
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 24.85	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724995.3 E 758992.6 N	Dates 22/02/2020	Engineer DBFL	Sheet 2/2

Depth (m)	Blows for Depth Increment	Field Records	Level (mOD)	Depth (m)	Blows for Depth Increment									
					0	5	10	15	20	25	30	35	40	45
5.00-5.10	14		19.85	5.00										
5.10-5.20	10													
5.20-5.30	10													
5.30-5.40	12													
5.40-5.50	14													
5.50-5.60	11		19.35	5.50										
5.60-5.70	8													
5.70-5.80	10													
5.80-5.90	15													
5.90-6.00	12													
6.00-6.10	15		18.85	6.00										
6.10-6.18	50													
			18.35	6.50										
			17.85	7.00										
			17.35	7.50										
			16.85	8.00										
			16.35	8.50										
			15.85	9.00										
			15.35	9.50										
			14.85	10.00										

Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP11



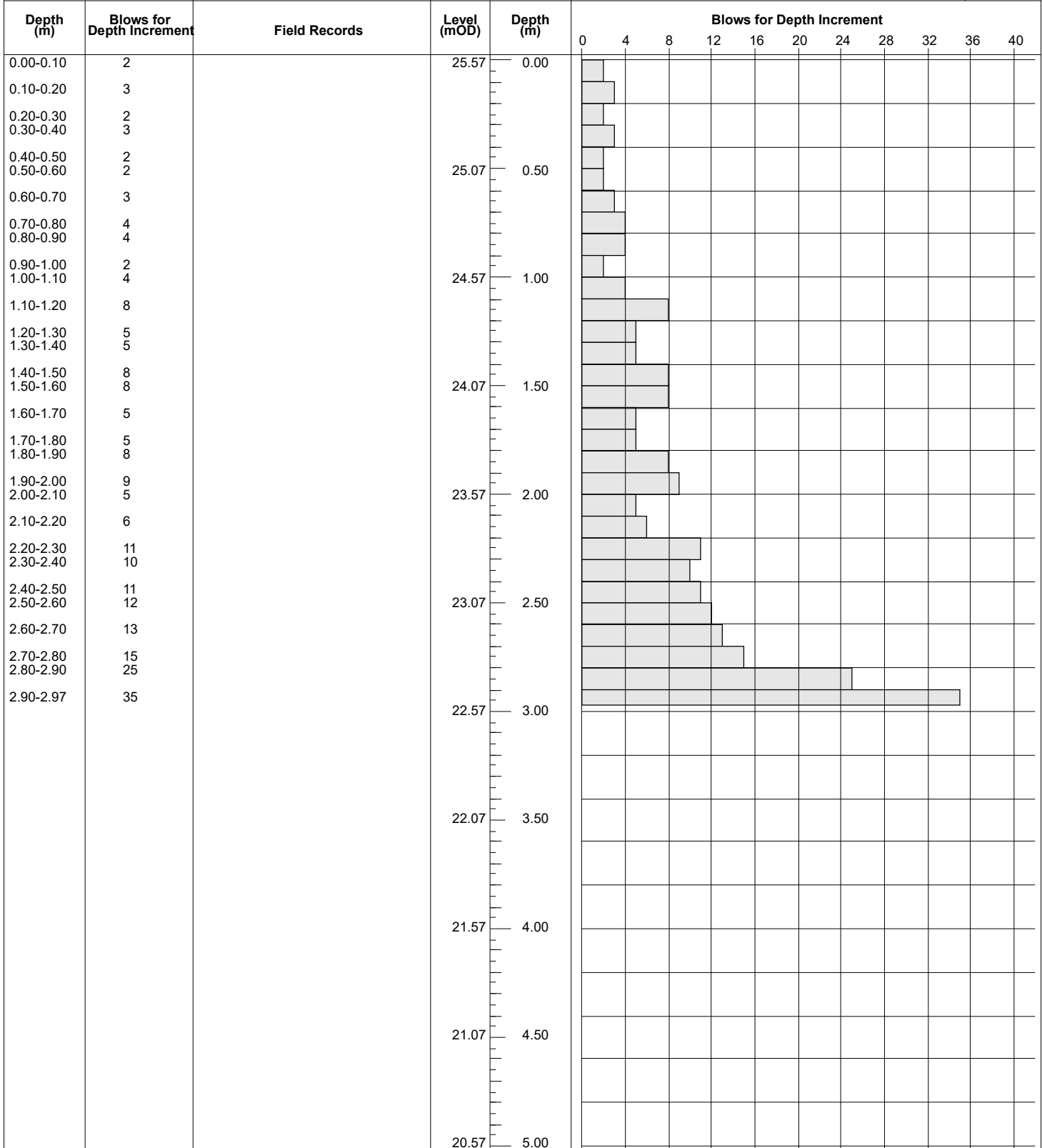
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 25.88	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724997 E 759069.6 N	Dates 21/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP12



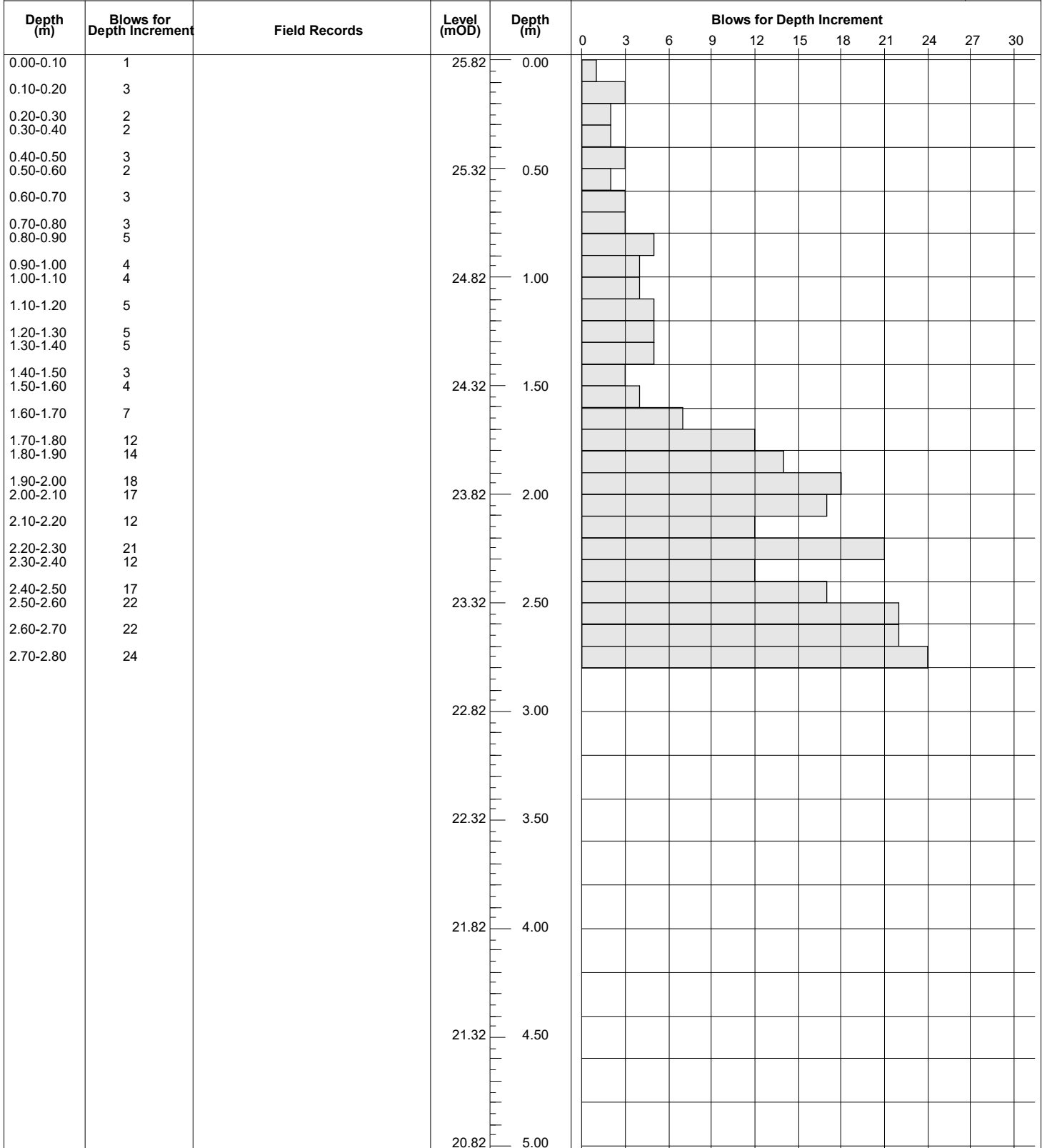
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 25.57	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725033.6 E 759081.3 N	Dates 21/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP13



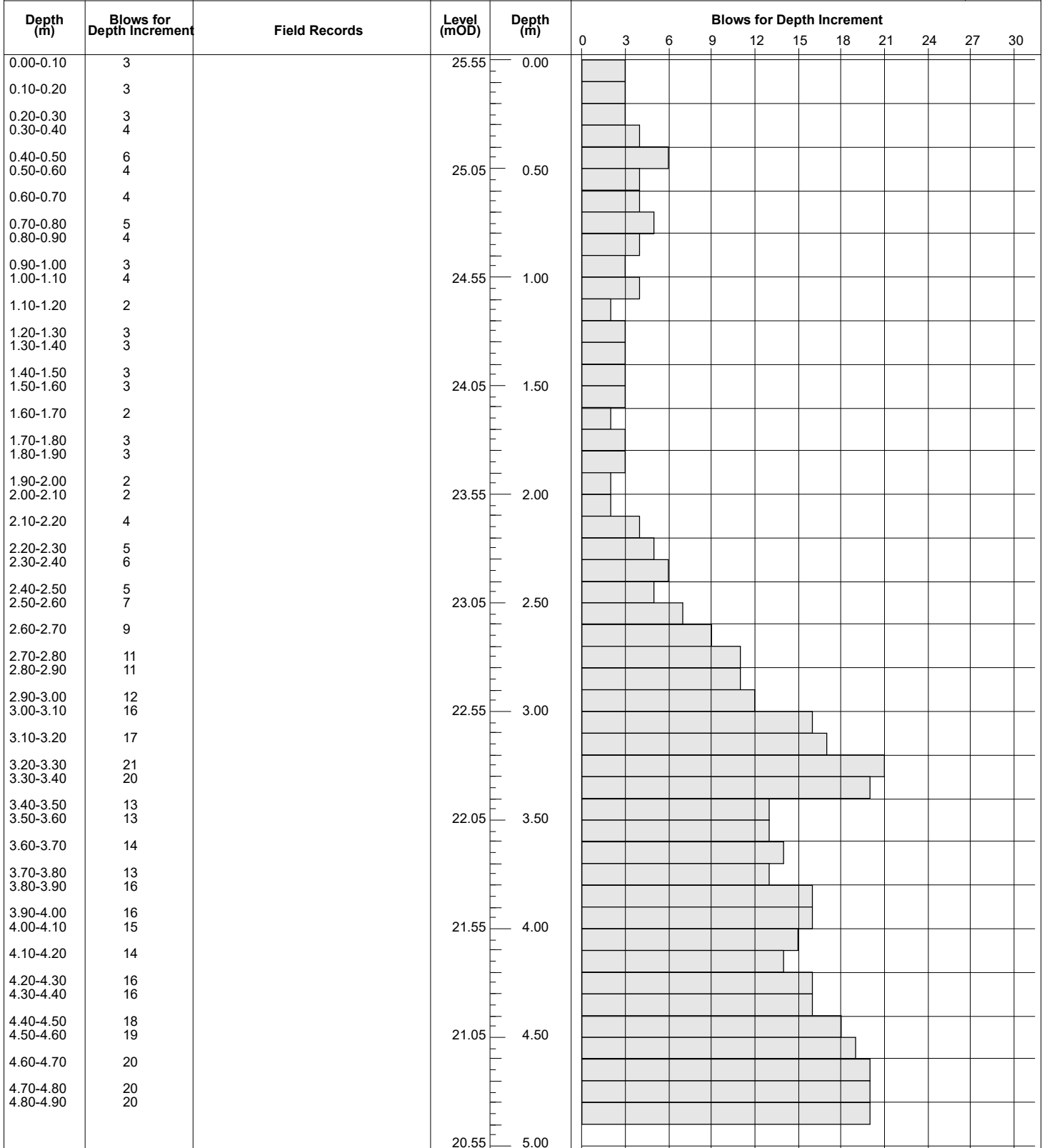
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 25.82	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725044 E 759049.5 N	Dates 21/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP14



Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 25.55	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725006.8 E 759034.3 N	Dates 21/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP15



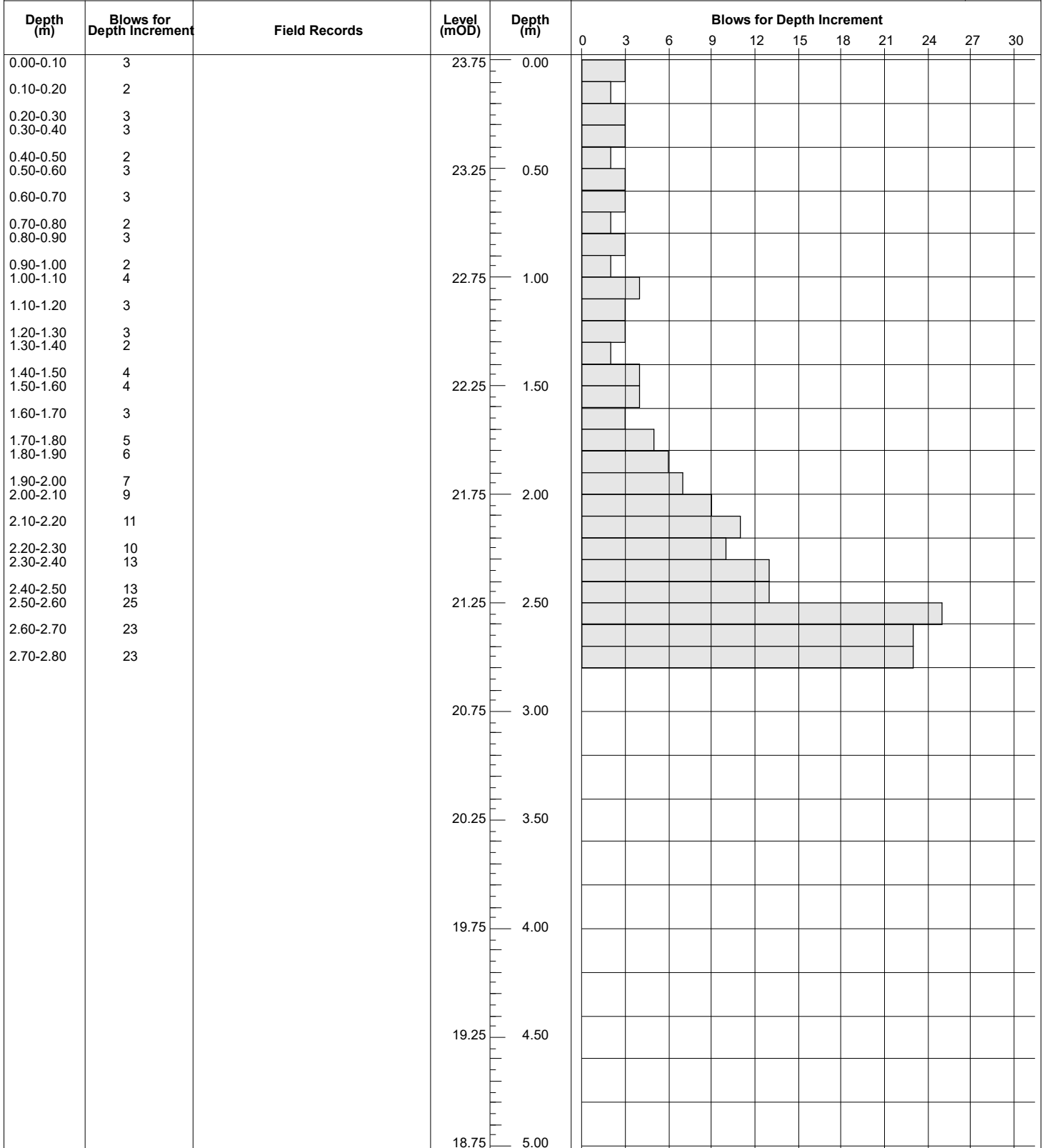
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 23.62	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725100.2 E 759039.3 N	Dates 21/02/2020	Engineer DBFL	Sheet 1/1

Depth (m)	Blows for Depth Increment	Field Records	Level (mOD)	Depth (m)	Blows for Depth Increment													
					0	3	6	9	12	15	18	21	24	27	30			
0.00-0.10	3		23.62	0.00														
0.10-0.20	3																	
0.20-0.30	2																	
0.30-0.40	4																	
0.40-0.50	4																	
0.50-0.60	3		23.12	0.50														
0.60-0.70	4																	
0.70-0.80	3																	
0.80-0.90	4																	
0.90-1.00	3																	
1.00-1.10	4		22.62	1.00														
1.10-1.20	4																	
1.20-1.30	3																	
1.30-1.40	3																	
1.40-1.50	2																	
1.50-1.60	5		22.12	1.50														
1.60-1.70	4																	
1.70-1.80	5																	
1.80-1.90	4																	
1.90-2.00	4																	
2.00-2.10	29		21.62	2.00														
2.10-2.20	20																	
2.20-2.30	24																	
				21.12	2.50													
				20.62	3.00													
				20.12	3.50													
				19.62	4.00													
				19.12	4.50													
				18.62	5.00													

Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP17



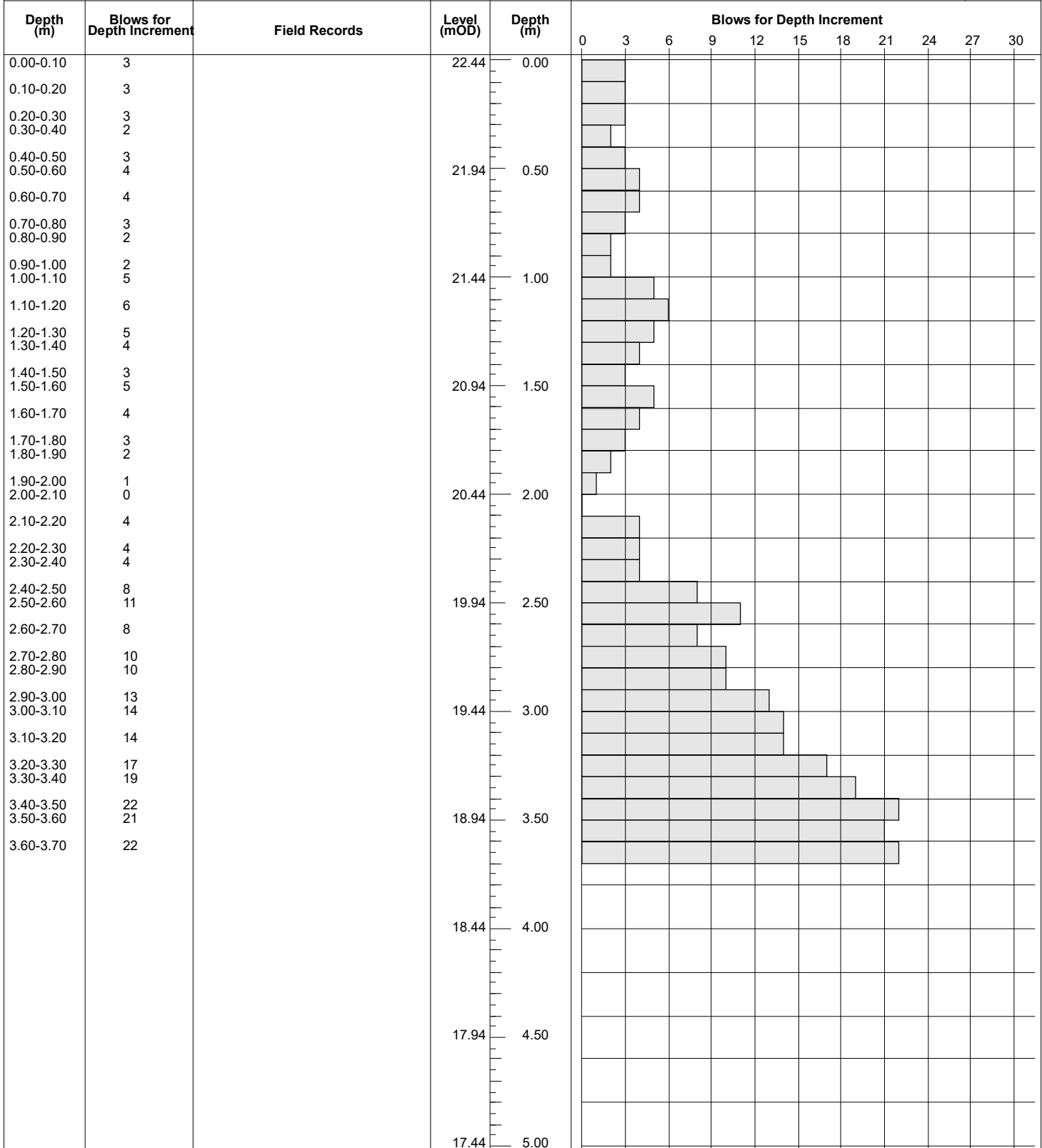
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 23.75	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725088.9 E 759079 N	Dates 21/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP18



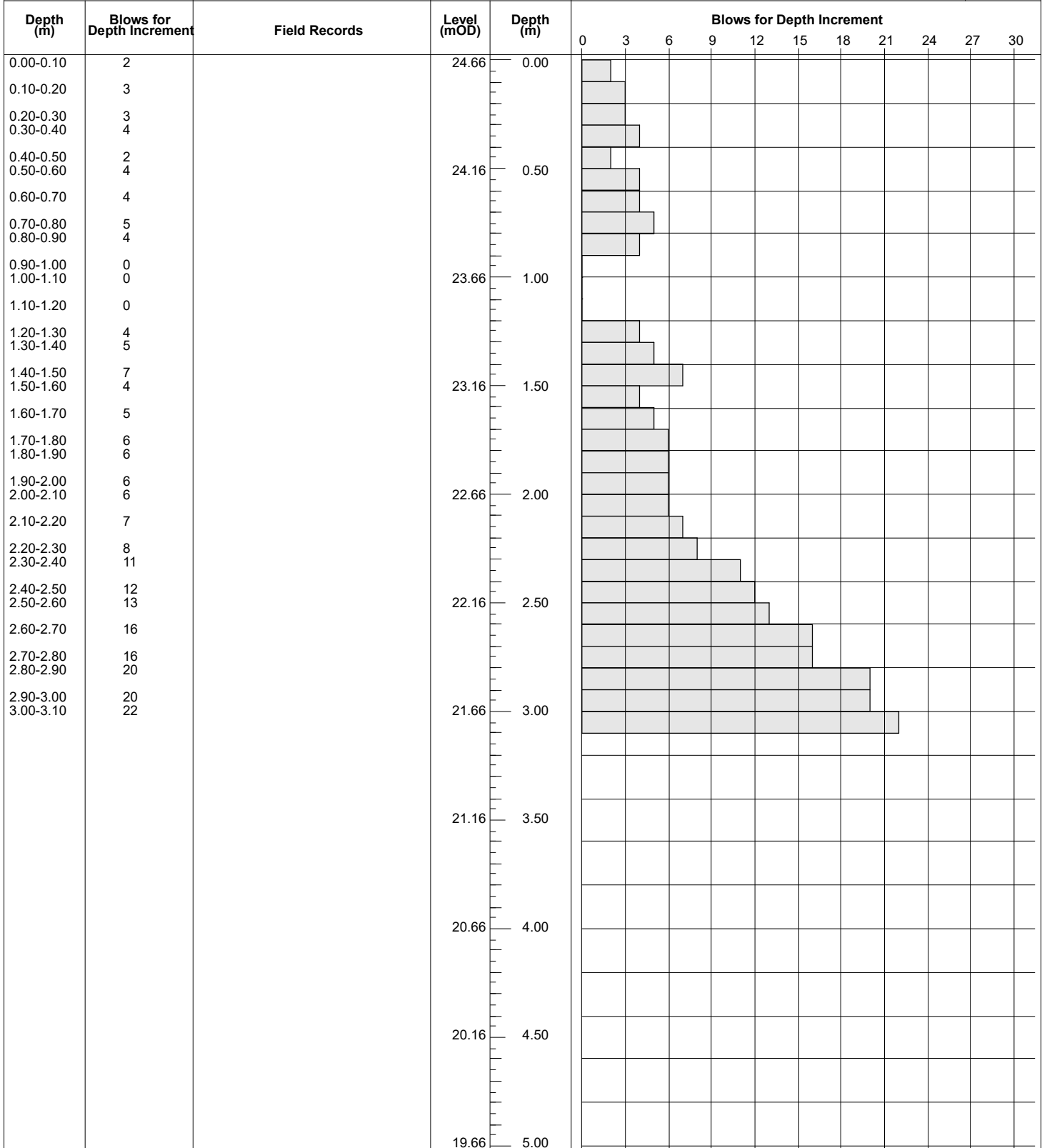
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 22.44	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725079.2 E 759116.1 N	Dates 21/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP19



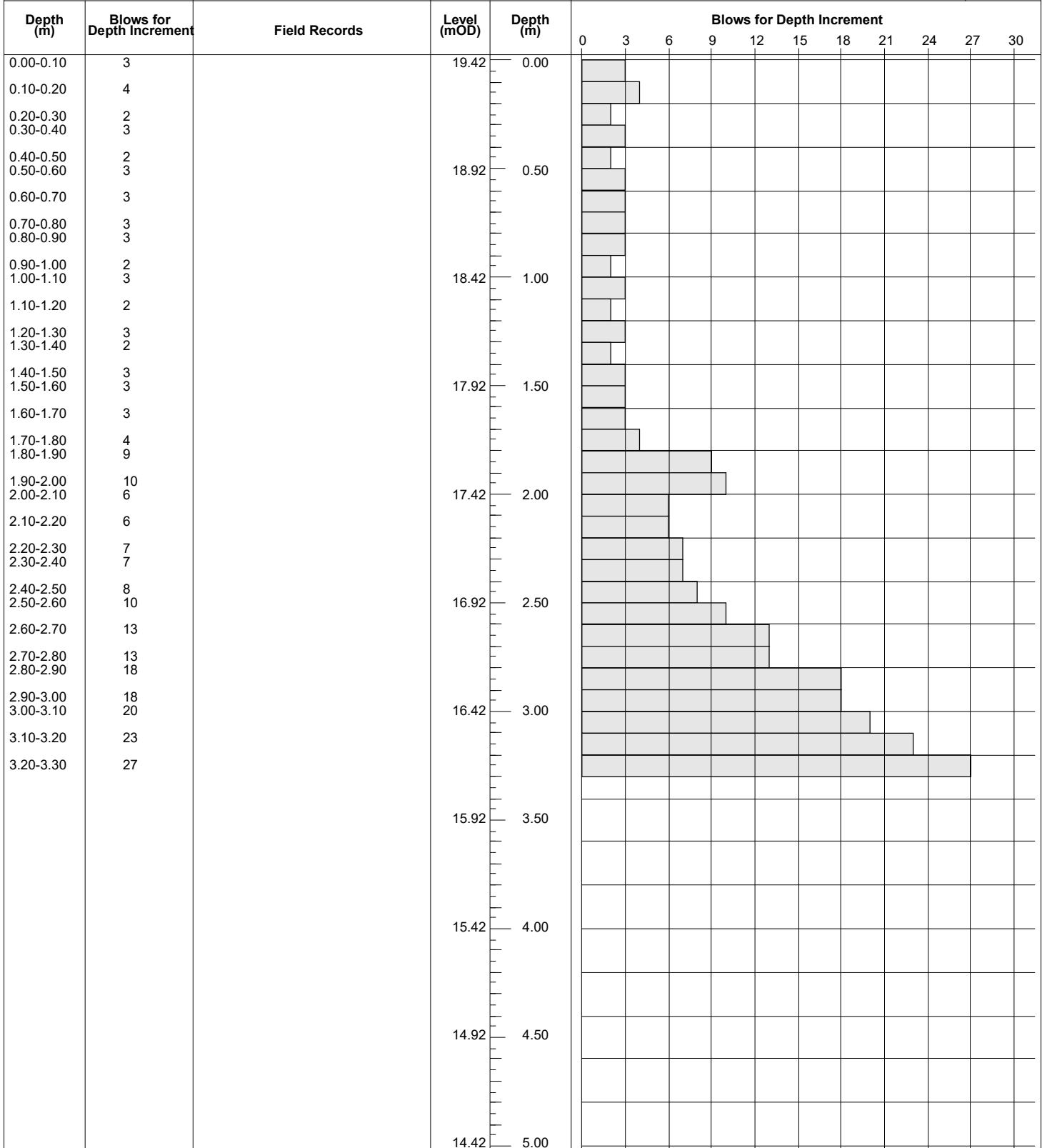
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 24.66	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724976.3 E 759020.8 N	Dates 21/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP20



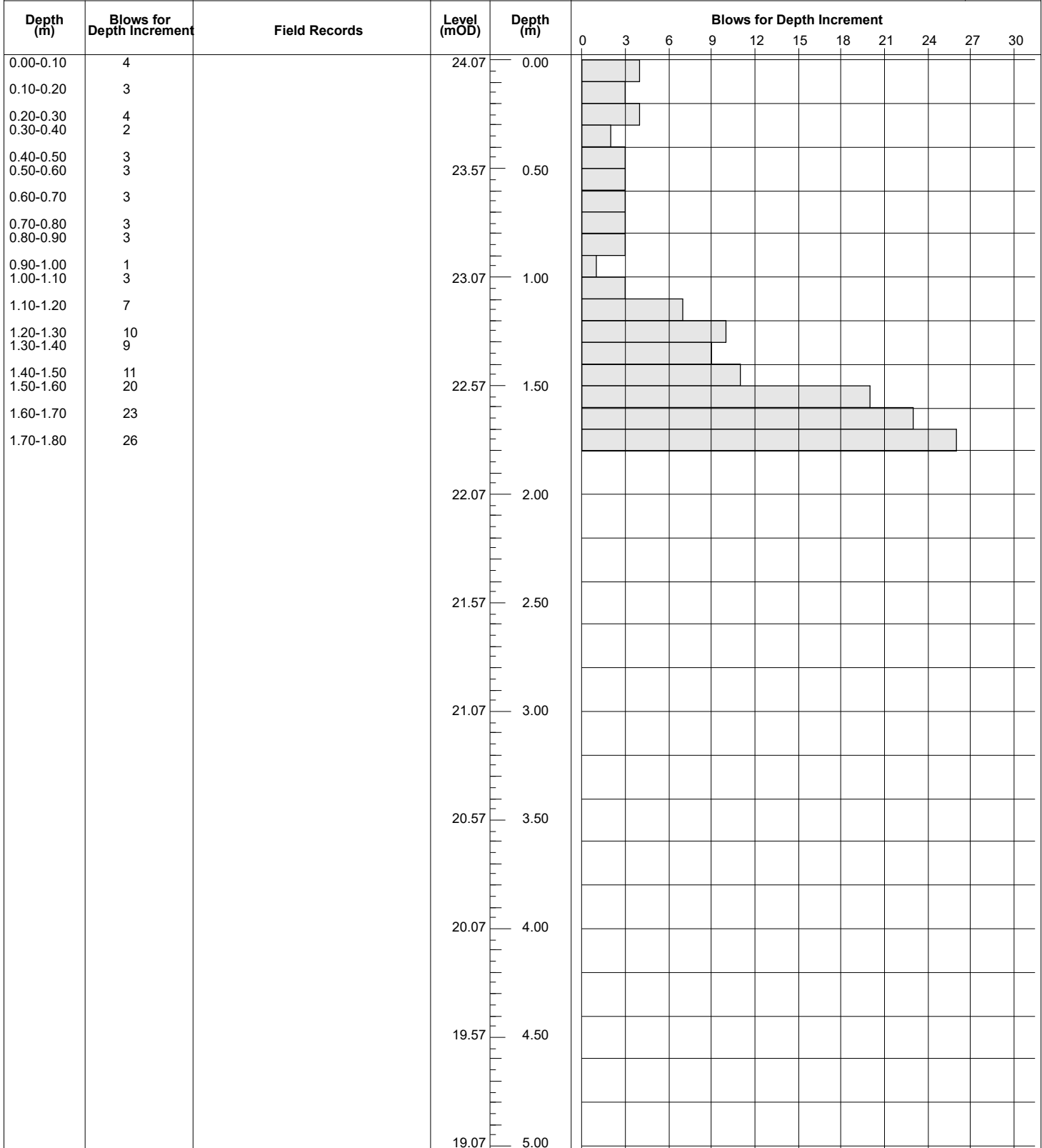
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 19.42	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725031.3 E 759203.7 N	Dates 22/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP21



Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 24.07	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724993.6 E 759153.7 N	Dates 22/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP22



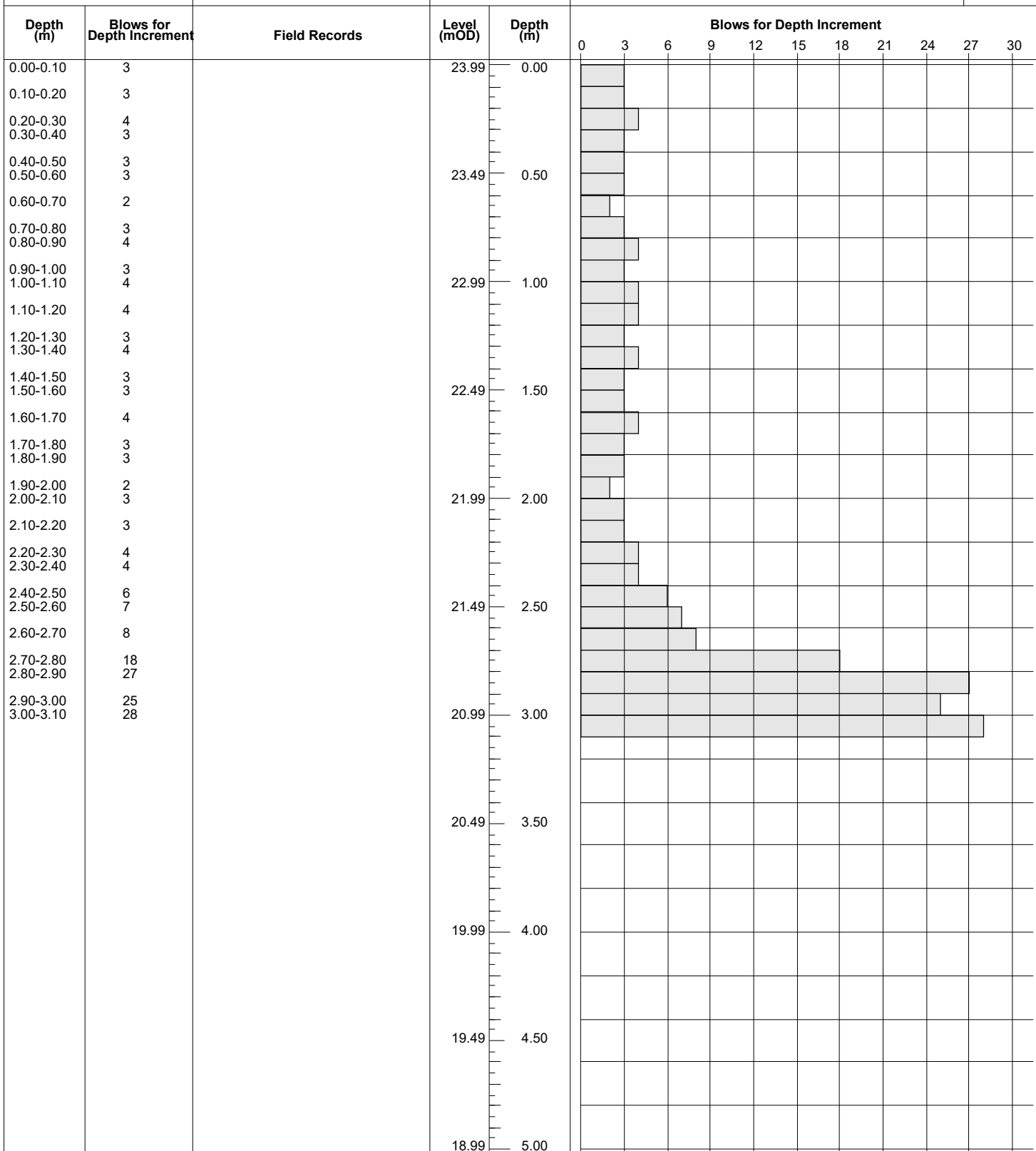
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 25.41	Client DBFL	Job Number 9225-11-19
	Location 725005.4 E 759110.7 N	Dates 22/02/2020	Engineer DBFL	Sheet 1/1

Depth (m)	Blows for Depth Increment	Field Records	Level (mOD)	Depth (m)	Blows for Depth Increment									
					0	5	10	15	20	25	30	35	40	45
0.00-0.10	4		25.41	0.00	[Bar chart showing 4 blows]									
0.10-0.20	3				[Bar chart showing 3 blows]									
0.20-0.30	2				[Bar chart showing 2 blows]									
0.30-0.40	3				[Bar chart showing 3 blows]									
0.40-0.50	3				[Bar chart showing 3 blows]									
0.50-0.60	4		24.91	0.50	[Bar chart showing 4 blows]									
0.60-0.70	4				[Bar chart showing 4 blows]									
0.70-0.80	4				[Bar chart showing 4 blows]									
0.80-0.90	3				[Bar chart showing 3 blows]									
0.90-1.00	2				[Bar chart showing 2 blows]									
1.00-1.10	3		24.41	1.00	[Bar chart showing 3 blows]									
1.10-1.20	2				[Bar chart showing 2 blows]									
1.20-1.30	2				[Bar chart showing 2 blows]									
1.30-1.40	3				[Bar chart showing 3 blows]									
1.40-1.50	2				[Bar chart showing 2 blows]									
1.50-1.60	3		23.91	1.50	[Bar chart showing 3 blows]									
1.60-1.70	4				[Bar chart showing 4 blows]									
1.70-1.80	9				[Bar chart showing 9 blows]									
1.80-1.90	11				[Bar chart showing 11 blows]									
1.90-2.00	15				[Bar chart showing 15 blows]									
2.00-2.10	17		23.41	2.00	[Bar chart showing 17 blows]									
2.10-2.20	17				[Bar chart showing 17 blows]									
2.20-2.25	50				[Bar chart showing 50 blows]									
			22.91	2.50	[Bar chart showing 0 blows]									
			22.41	3.00	[Bar chart showing 0 blows]									
			21.91	3.50	[Bar chart showing 0 blows]									
			21.41	4.00	[Bar chart showing 0 blows]									
			20.91	4.50	[Bar chart showing 0 blows]									
			20.41	5.00	[Bar chart showing 0 blows]									

Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP23



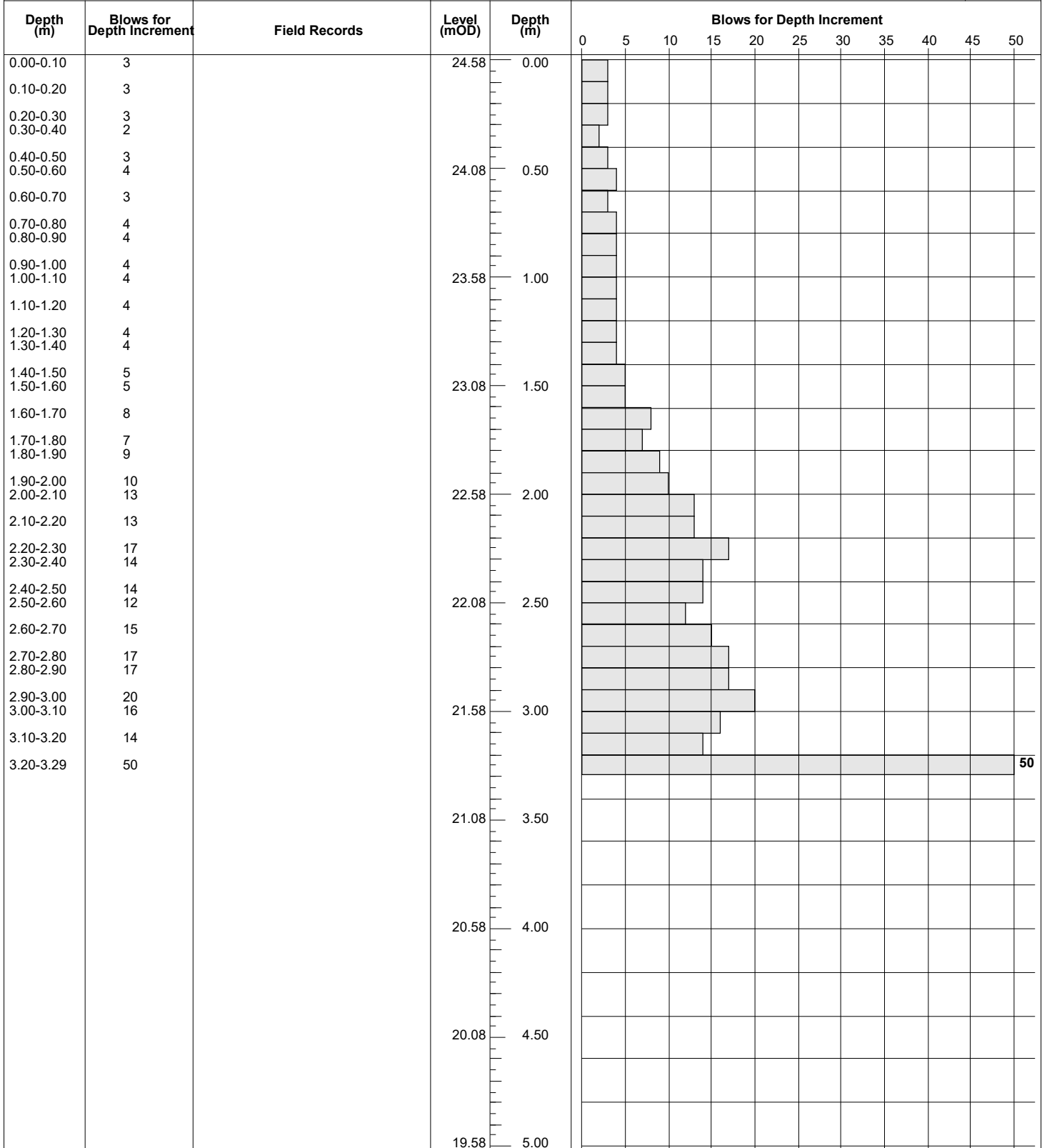
Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 23.99	Client DBFL	Job Number 9225-11-19
	Location 725047.5 E 759115.1 N	Dates 21/02/2020- 21/03/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP24



Method Dynamic Probe	Cone Dimensions	Ground Level (mOD) 24.58	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724958.5 E 759055.3 N	Dates 22/02/2020	Engineer DBFL	Sheet 1/1



Remarks	Scale (approx)	Logged By
	1:25	
	Figure No.	9225-11-19.DP25

APPENDIX 5 – Cable Percussion and Rotary Borehole Records





Machine : Dando 1500 Method : Cable Percussion	Casing Diameter 200mm cased to 7.10m	Ground Level (mOD) 19.35	Client DBFL	Job Number 9225-11-19
Location 725040.3 E 759288.1 N		Dates 12/03/2020- 13/03/2020	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.40	B				19.20	(0.15)	FILL. Grey sandy angular medium to coarse gravel (crushed rock fill).		
0.70	B				18.65	(0.55)	MADE GROUND. Reddish brown slightly gravelly sandy silty Clay with occasional subangular to sub rounded cobbles and brick and glass fragments. Gravel is fine to coarse subangular to subrounded.		
1.00-1.38	SPT(C) 50/225			7,13/17,14,16,3	18.35	1.00	MADE GROUND Reddish brown sandy gravelly CLAY with many subangular to subrounded cobbles. Gravel is fine to coarse subangular to subrounded.		
1.50	B						MADE GROUND brown slightly sandy gravelly CLAY with many subangular to subrounded cobbles. Gravel is fine to coarse subangular to subrounded.		
2.00-2.45	SPT(C) N=44			5,7/9,16,8,11		(2.20)			
2.50	B								
3.00-3.45	SPT(C) N=11			8,8/4,2,2,3	16.15	3.20	Firm brown/grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles and woody fragments. Gravel fine to coarse subangular to subrounded.		
3.30	B				15.75	3.60	Firm brown slightly sandy gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is fine to coarse subangular to subrounded.		
4.00-4.45	SPT(C) N=10			2,2/3,2,2,3					
4.50	B					(2.00)			
5.00-5.45	SPT(C) N=9			2,1/2,2,2,3					
5.70	B				13.75	5.60	Medium dense sandy GRAVEL with occasional subangular to subrounded cobbles. Gravel is fine to coarse subangular to subrounded.		▽1
6.00-6.45	SPT(C) N=34			Water strike(1) at 5.90m, rose to 5.20m in 20 mins. 3,6/6,7,9,12		(1.50)			▽1
6.50	B								
7.10-7.30	SPT(C) 50/0			20/50	12.25	7.10	Complete at 7.10m		

Remarks Borehole backfilled on completion. Chiselling from 7.10m to 7.10m for 1 hour.	Scale (approx)	Logged By
	1:50	CB
	Figure No. 9225-11-19.BH01	



Machine : Dando 2000 & Beretta T44 Method : Cable Percussion and Rotary Core Follow on	Casing Diameter 200mm cased to 5.00m 96mm cased to 12.70m	Ground Level (mOD) 15.80	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725029.5 E 759251.3 N	Dates 26/02/2020- 26/03/2020	Project Contractor Ground Investigations Ireland	Sheet 1/2

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.90 1.00-1.45	B SPT(C) N=4			0,1/0,1,1,2	15.30	(0.50) 0.50	Brown slightly sandy slightly gravelly TOPSOIL. Gravel is subrounded, fine to coarse. Sand is fine to coarse.		
2.00 2.00-2.45	B SPT(C) N=12			1,0/2,2,4,4	14.10	(1.20) 1.70	Soft greyish brown slightly gravelly sandy CLAY with rootlets and timber fragments. Gravel is subrounded, fine to coarse. Sand is fine to coarse (Possible Made Ground).		
3.00 3.00-3.45	B SPT(C) N=12			4,5/4,4,3,1		(2.10)	Medium dense brownish grey slightly clayey sandy subangular to subrounded, fine to coarse GRAVEL with occasional sub-rounded cobbles. Sand is fine to coarse.		▼1 ▽1
4.00 4.00-4.45	B SPT(C) N=32			6,6/7,7,8,10	12.00	(1.20) 3.80	Stiff grey slightly sandy slightly gravelly CLAY. Gravel is subrounded, fine to coarse. Sand is fine to coarse.		
5.00-5.35 5.00	TCR 29	SCR	RQD	FI 8,10/19,25,6 SPT(C) 50/200	10.80	5.00	Stiff dark brownish grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse. 5.00-6.00m BGL: Driller's Notes - Clay with occasional boulders		
6.70-7.15 6.70	83			2,6/6,5,6,6 SPT(C) N=23		(4.50)			
8.20-8.65 8.20	51			4,4/5,6,6,7 SPT(C) N=24					
9.70-9.85 9.70 9.80				6,19/50 SPT(C) 25*/140 50/10	6.30 6.00	9.50 (0.30) 9.80	Very stiff brown slightly sandy gravelly CLAY with occasional subangular cobbles. Gravel is angular to subrounded fine to coarse		

Remarks Borehole backfilled on completion. Cable percussion refusal at 5.00m BGL due to obstruction, possible boulder. Rotary Core follow on from 5.00m BGL Chiselling from 5.35m to 5.35m for 0.5 hours.	Scale (approx) 1:50	Logged By JS & PC
	Figure No. 9225-11-19.BH02	



Machine : Dando 2000 & Beretta T44 Flush : Water Core Dia : 63.5 mm Method : Cable Percussion and Rotary Core Follow on	Casing Diameter 200mm cased to 5.00m 96mm cased to 12.70m	Ground Level (mOD) 15.80	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725029.5 E 759251.3 N	Dates 26/02/2020- 26/03/2020	Project Contractor Ground Investigations Ireland	Sheet 2/2

Depth (m)	TCR	SCR	RQD	FI	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
10.80	100	54	48	NI		5.00	(1.00)	Medium strong grey fine grained LIMESTONE with occasional calcite veining. Partially to distinctly weathered 9.80-10.80m - Mostly Non Intact		
11.20				6		4.10	(0.90)	Medium strong to strong grey fine grained LIMESTONE with occasional calcite veining. Partially to distinctly weathered 10.80-11.70m - Two fracture sets. F1: Closely to medium spaced, 0 to 10°, stepped rough with clay infilling. F2: Medium spaced, 60° to 80°, undulating rough		
11.70	100	94	88	2		3.10	(1.00)	Strong grey fine grained LIMESTONE with occasional calcite veining. Partially weathered 11.70-12.70m - F1: Medium spaced, 60° to 80°, undulating rough with clay infilling		
12.70								Complete at 12.70m		

Remarks	Scale (approx)	Logged By
	1:50	JS & PC
	Figure No. 9225-11-19.BH02	



Machine : Dando 2000	Casing Diameter 200mm cased to 6.80m	Ground Level (mOD) 16.41	Client DBFL	Job Number 9225-11-19
Method : Cable Percussion	Location (dGPS) 724940.3 E 759259.9 N	Dates 25/02/2020- 26/02/2020	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
1.00-1.45 1.00	SPT(C) N=9 B			3,4/2,2,2,3	16.11	(0.30) 0.30	Brown slightly sandy slightly gravelly TOPSOIL. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.			
						(1.20)	Soft to firm brown slightly gravelly sandy CLAY with rootlets and occasional sub-rounded cobbles. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse (Possible Made Ground).			
2.00-2.45 2.00	SPT(C) N=9 B			2,3/2,2,3,2	14.91	1.50	Soft brown gravelly sandy CLAY with rootlets and occasional sub-rounded cobbles. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.			
					14.51	1.90	Firm brownish grey slightly sandy slightly gravelly CLAY with occasional sub-rounded cobbles and boulders. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.			
3.00	B			Water strike(1) at 2.90m, rose to 2.70m in 20 mins.	13.41	3.00	Stiff brownish grey slightly sandy slightly gravelly CLAY with occasional sub-rounded cobbles and boulders. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.		▼1	
3.00-3.45	SPT(C) N=17			2,3/9,3,2,3		(1.70)				
4.00-4.45	SPT(C) N=32			3,4/6,10,8,8						
4.70	B				11.71	4.70	Brownish grey slightly sandy clayey GRAVEL. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse. (Driller notes pockets of Clay)			
5.00-5.45	SPT(C) N=21			17,6/4,5,5,7	11.41	5.00	Stiff brownish grey slightly gravelly sandy CLAY. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.			
5.50	B					(1.10)				
6.10	B				10.31	6.10	Firm greyish brown slightly sandy slightly gravelly CLAY. Gravel is sub-angular to sub-rounded, fine to medium. Sand is fine to coarse.			
6.40	B				10.01	6.40	Firm to stiff brown slightly sandy gravelly CLAY with occasional sub-angular to sub-rounded cobbles. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.			
6.50-6.80	SPT(C) 50/150			14,7/5,5,40	9.61	6.80	Refusal at 6.80m			

Remarks Borehole backfilled on completion. Refusal at 6.80m BGL due to obstruction, possible rock or boulder. Slotted pipe with pea gravel surround from 6.8m BGL to 1.0m BGL, plain pipe with bentonite seal from 1.0m BGL to ground level, finished with an upright cover. Chiselling from 5.00m to 5.00m for 0.25 hours.	Scale (approx)	Logged By
	1:50	JS
	Figure No. 9225-11-19.BH03	



Machine : Dando 2000 & Beretta T44 Method : Cable Percussion and Rotary Follow On	Casing Diameter 200mm cased to 3.20m 96mm cased to 12.70m	Ground Level (mOD) 19.92	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724984.4 E 759214.9 N	Dates 21/02/2020-25/03/2020	Project Contractor Ground Investigations Ireland	Sheet 1/2

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
1.00 1.00-1.45	B SPT(C) N=6			1,1/1,1,2,2	19.77	(0.15) 0.19	Brown slightly sandy slightly gravelly TOPSOIL. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.		
1.80 2.00-2.45	B SPT(C) N=30			4,4/6,7,8,9	18.22 17.92	(1.55) 1.70 (0.30) 2.00	Soft reddish brown sandy gravelly silty CLAY with occasional sub-rounded cobbles. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse. Loose brown clayey sandy sub rounded fine to coarse GRAVEL with occasional sub-rounded cobbles and boulders. Sand is fine to coarse.		
2.50 3.00-3.20	B SPT(C) 50/50			10,22/50		(1.20)	Medium dense grey slightly clayey sandy sub-rounded, fine to coarse GRAVEL with occasional sub-rounded cobbles. Sand is fine to coarse.		
3.20	TCR 80	SCR	RQD	FI	16.72	3.20 (0.35)	Stiff brown slightly sandy gravelly CLAY with occasional subrounded cobbles. Gravel is subangular to subrounded		
3.70-4.15 3.70	50			5,5/5,5,7,8 SPT(C) N=25	16.37	3.55 (1.35)	Recovery consists of medium dense grey subangular to subrounded fine to coarse GRAVEL with occasional subrounded cobbles Driller notes clay and cobbles.		
5.20-5.65 5.20	70			6,7/6,6,7,9 SPT(C) N=28	15.02	4.90 (1.80)	Stiff greyish brown slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to coarse		
6.70-7.15 6.70	77			8,9/9,10,12,12 SPT(C) N=43	13.22	6.70 (0.60)	Very stiff greyish brown slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to coarse		
8.20-8.35 8.20	48			12,13/50 SPT(C) 25*/140 50/10	12.62	7.30 (3.40)	Very stiff brown slightly sandy gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse		
9.70									

Remarks Borehole backfilled on completion. Refusal at 3.20m BGL due to obstruction, possible boulder. Rotary Core follow on from 3.20m BGL Chiselling from 3.20m to 3.20m for 0.6 hours.	Scale (approx) 1:50	Logged By JS & PC
	Figure No. 9225-11-19.BH04	



Machine : Dando 2000 & Beretta T44 Flush : Water Core Dia : 63.5 mm Method : Cable Percussion and Rotary Follow On	Casing Diameter 200mm cased to 3.20m 96mm cased to 12.70m	Ground Level (mOD) 19.92	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724984.4 E 759214.9 N	Dates 21/02/2020-25/03/2020	Project Contractor Ground Investigations Ireland	Sheet 2/2

Depth (m)	TCR	SCR	RQD	FI	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
10.70	83			NI		9.22	10.70 (0.60)	Medium strong orange pinkish white medium grained LIMESTONE with occasional calcite veining and frequent vugs. Distinctly weathered 10.70-11.30m - Mostly Non Intact		
11.20 11.30	100	71	65	5		8.62	11.30 (1.40)	Strong orange pinkish white medium grained LIMESTONE with occasional calcite veining and frequent vugs. Partially to distinctly weathered 11.30-12.70m - F1: Closely to medium spaced, 0° to 10°, stepped rough		
12.70						7.22	12.70	Complete at 12.70m		

Remarks	Scale (approx)	Logged By
	1:50	JS & PC
	Figure No. 9225-11-19.BH04	



Machine : Dando 2000	Casing Diameter 200mm cased to 2.60m	Ground Level (mOD) 20.94	Client DBFL	Job Number 9225-11-19
Method : Cable Percussion	Location (dGPS) 725019.1 E 759188.7 N	Dates 27/02/2020	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
1.00-1.45 1.00	SPT(C) N=6 B			1,0/1,0,2,3	20.74	(0.20) 0.20	Brown slightly sandy slightly gravelly TOPSOIL. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.		
2.00-2.38 2.00	SPT(C) 50/230 B			6,11/13,20,17	18.94	(1.80) 2.00	Soft brown slightly sandy gravelly CLAY. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse (Possible Made Ground).		▼1
2.60-2.62	SPT(C) 25*/20 50/0			Water strike(1) at 2.40m, rose to 2.00m in 20 mins. 25/50	18.34	(0.60) 2.60	Stiff brown slightly gravelly sandy CLAY with occasional sub-rounded cobbles. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.		▽1
							Refusal at 2.60m		

Remarks Borehole backfilled on completion. Refusal at 2.60m BGL due to obstruction. Chiselling from 2.60m to 2.60m for 0.3 hours.	Scale (approx)	Logged By
	1:50	JS
	Figure No. 9225-11-19.BH05	



Machine : Dando 2000	Casing Diameter 200mm cased to 4.60m	Ground Level (mOD) 21.32	Client DBFL	Job Number 9225-11-19
Method : Cable Percussion	Location (dGPS) 725018.9 E 759185.4 N	Dates 27/02/2020- 28/02/2020	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
1.00-1.45 1.00	SPT(C) N=3 B			0,1/1,0,1,1	21.07	(0.25) 0.25	Brown slightly sandy slightly gravelly TOPSOIL. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.			
						(1.55)	Very soft brown sandy gravelly CLAY with some rootlets. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.			
2.00-2.45 2.00	SPT(C) N=39 B			4,6/8,9,10,12	19.52	1.80	Stiff greyish brown sandy gravelly CLAY with occasional sub-rounded cobbles and boulders. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.			
				Water strike(1) at 2.80m, rose to 2.70m in 20 mins.		(1.20)				
3.00-3.32 3.10	SPT(C) 50/170 B			6,10/14,20,16	18.32	3.00	Medium dense to dense brown clayey gravelly fine to coarse SAND. Gravel is sub-angular to sub-rounded, fine to coarse.			
						(1.00)				
4.00-4.35 4.00 4.30	SPT(C) 50/200 B B			7,7/7,11,32	17.32	4.00	Medium dense to dense brown silty sandy sub-angular to sub-rounded, fine to coarse GRAVEL. Sand is fine to coarse.			
					17.02	(0.30)				
4.60-4.60	SPT(C) 25*/0 50/0			25/50	16.72	4.60	Stiff brownish grey slightly sandy slightly gravelly silty CLAY. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse.			
							Refusal at 4.60m			

Remarks Borehole backfilled on completion. Refusal at 4.60m BGL due to obstruction. Possible boulder or rock. Slotted pipe with pea gravel surround from 4.6m BGL to 1.0m BGL, plain pipe with bentonite seal from 1.0m BGL to ground level, finished with an upright cover. Chiselling from 4.60m to 4.60m for 0.5 hours.	Scale (approx)	Logged By
	1:50	JS
	Figure No. 9225-11-19.BH05A	



Machine : Dando 2000 Method : Cable Percussion	Casing Diameter 200mm cased to 4.00m	Ground Level (mOD) 20.92	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725062.5 E 759157 N	Dates 20/02/2020- 21/02/2020	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
1.00-1.45 1.00	SPT(C) N=6 B			0,1/1,2,1,2	20.62	(0.30) 0.30	Brown slightly sandy slightly gravelly TOPSOIL. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.		
2.00-2.45 2.20	SPT(C) N=27 B			2,3/5,6,8,8 Water strike(1) at 2.30m, rose to 2.20m in 20 mins.	18.82	(1.80) 2.10	Soft brown slightly sandy slightly gravelly CLAY with rootlets. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse (Possible Made Ground).		
3.00-3.25 3.10	SPT(C) 50/100 B			10,13/25,25	17.92	(0.90) 3.00	Stiff brown slightly sandy slightly gravelly CLAY with occasional sub-rounded boulders. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.		▼1
4.00-4.00	SPT(C) 25*/0 50/0			Water strike(2) at 4.00m, rose to 3.85m in 20 mins. 25/50	16.92	(1.00) 4.00	Medium dense to dense brown clayey sandy GRAVEL with occasional sub-rounded cobbles. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.		▼2
							Refusal at 4.00m		▼2

Remarks Borehole backfilled on completion. Refusal at 4.00m BGL due to obstruction, possible boulder or rock. Chiselling from 3.70m to 3.70m for 0.3 hours. Chiselling from 4.00m to 4.00m for 0.5 hours.	Scale (approx)	Logged By
	1:50	JS
	Figure No. 9225-11-19.BH06	



Machine : Dando 2000 Method : Cable Percussion	Casing Diameter 200mm cased to 7.10m	Ground Level (mOD) 19.02	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724929.7 E 759222.4 N	Dates 24/02/2020- 25/02/2020	Project Contractor Ground Investigations Ireland	Sheet 1/1

Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
1.00-1.45 1.00	SPT(C) N=8 B			1,2/2,1,2,3	18.52	(0.50) 0.50	Brown slightly sandy slightly gravelly TOPSOIL. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.			
2.00-2.45 2.00	SPT(C) N=6 B			2,2/2,1,1,2 Water strike(1) at 2.10m, rose to 1.85m in 20 mins.	17.42	(1.10) 1.60	Soft brown slightly sandy slightly gravelly CLAY with rootlets. Gravel is sub-angular to sub-rounded, fine to coarse. Sand is fine to coarse (Possible Made Ground).			
3.00-3.45 3.00	SPT(C) N=11 B			1,2/2,2,3,4	16.02	(0.40) 3.00	Firm brown slightly sandy slightly gravelly silty CLAY. Gravel is sub-rounded, fine to medium. Sand is fine to coarse.			
3.50	B				15.62	(0.50)	Firm to stiff reddish brown slightly sandy slightly gravelly silty CLAY. Gravel is sub-rounded, fine to coarse. Sand is fine to coarse.			
4.00-4.45 4.20	SPT(C) N=34 B			9,7/5,9,10,10	15.12	(1.90) 3.90	Medium dense brown slightly clayey slightly sandy sub-angular to sub-rounded, fine to coarse GRAVEL with occasional sub-angular to sub-rounded cobbles. Sand is fine to coarse. (Driller notes occasional pockets of Clay)			
5.00-5.45 5.20	SPT(C) N=30 B			5,5/6,7,9,8	13.22	(1.30) 5.80	Stiff reddish brown slightly sandy slightly gravelly CLAY. Gravel is sub-rounded, fine to medium. Sand is fine to coarse.			
6.20 6.50-6.95	SPT(C) N=29			6,5/6,6,8,9	11.92	(1.30) 7.10	Refusal at 7.10m			

Remarks Borehole backfilled on completion. Refusal at 7.10m BGL due to obstruction, possible boulder or rock. Slotted pipe with pea gravel surround from 7.0m BGL to 1.0m BGL, plain pipe with bentonite seal from 1.0m BGL to ground level, finished with an upright cover. Chiselling from 7.10m to 7.10m for 0.6 hours.	Scale (approx)	Logged By
	1:50	JS
Figure No. 9225-11-19.BH07		



Machine : Beretta T44 Flush : Water Core Dia: 63.5 mm Method : Rotary Cored	Casing Diameter 96mm cased to 12.70m	Ground Level (mOD) 19.02	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725054.4 E 759189.6 N	Dates 26/03/2020	Project Contractor Ground Investigations Ireland	Sheet 1/2

Depth (m)	TCR	SCR	RQD	FI	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
1.00 1.00-1.45	93	-			2,2/3,3,4,3 SPT(C) N=13	18.82	(0.20) 0.20	TOPSOIL Soft to Firm brown slightly sandy slightly gravelly CLAY. Gravel is subangular to subrounded fine to coarse		
2.20 2.20-2.65	60	-			2,3/3,4,4,5 SPT(C) N=16	18.02	1.00 (1.20)	Firm to stiff brown slightly sandy slightly gravelly CLAY. Gravel is subangular to subrounded fine to coarse		
3.70 3.70-4.15	59	-			4,7/7,8,7,9 SPT(C) N=31	16.82	2.20 (0.90)	Stiff brown slightly sandy slightly gravelly CLAY. Gravel is subangular to subrounded fine to coarse		
5.20 5.20-5.65	47	-			9,10/9,11,11,12 SPT(C) N=43	15.92	3.10 (0.60)	Stiff light brown slightly sandy gravelly CLAY with occasional subangular cobbles. Gravel is subangular to subrounded fine to coarse		
6.70 6.70-7.15	23	-			10,9/10,11,11,10 SPT(C) N=42	15.32	3.70 (3.30)	Very stiff grey slightly sandy gravelly CLAY with occasional subangular cobbles. Gravel is subangular to subrounded fine to coarse		
8.20 8.20-8.36	65	-			11,14/50 SPT(C) 50/10	12.02	7.00	Very stiff brown slightly sandy gravelly CLAY with occasional subangular cobbles and boulders of Limestone. Gravel is angular to subrounded fine to coarse		
9.70 9.70-10.08	75	-			12,12/14,15,21 SPT(C) 50/225		(4.10)			

Remarks Borehole backfilled on completion.	Scale (approx)	Logged By
	1:50	PC & CB
	Figure No. 9225-11-19.RC08	



Machine : Beretta T44 Flush : Water Core Dia: 63.5 mm Method : Rotary Cored	Casing Diameter 96mm cased to 12.70m	Ground Level (mOD) 19.02	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 725054.4 E 759189.6 N	Dates 26/03/2020	Project Contractor Ground Investigations Ireland	Sheet 2/2

Depth (m)	TCR	SCR	RQD	FI	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
11.10 11.20	80	80	0			7.92	11.10	Strong grey fine grained LIMESTONE with some calcite veining. Partially weathered. One set of fractures. F1 5-15 degrees. Closely spaced undulating smooth closed.		
	93	75	68	5			(1.60)			
12.70						6.32	12.70	Complete at 12.70m		

Remarks	Scale (approx)	Logged By
	1:50	PC & CB
Figure No. 9225-11-19.RC08		



Machine : Beretta T44 Flush : Water Core Dia: 63.5 mm Method : Rotary Cored	Casing Diameter 96mm cased to 15.50m	Ground Level (mOD) 25.01	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724967.4 E 759062.1 N	Dates 26/03/2020- 27/03/2020	Project Contractor Ground Investigations Ireland	Sheet 1/2

Depth (m)	TCR	SCR	RQD	FI	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
0.00	-	-				24.81	(0.20) 0.20	TOPSOIL			
2.20 2.20-2.65	86	-			2,2/3,3,4,6 SPT(C) N=16	22.81	2.20 (1.50)	Stiff brown slightly sandy gravelly CLAY with occasional cobbles of limestone. Gravel is subangular-subrounded fine-coarse.			
3.70 3.70-4.15	83	-			8,10/12,11,12,14 SPT(C) N=49	21.31	3.70 (2.70)	Very stiff brown slightly sandy gravelly CLAY with occasional cobbles of limestone. Gravel is subangular-subrounded fine-coarse.			
5.20 5.20-5.65	67	-			10,11/10,12,14,14 SPT(C) N=50	18.61	6.40	Poor Recovery. Recovery consists of slightly sandy clayey GRAVEL. Gravel is subangular-subrounded fine-coarse. Driller notes: gravelly Clay.			
6.70 6.70-7.15	20	-			8,8/9,10,12,16 SPT(C) N=47						
8.20 8.20-8.65	27	-			7,11/12,10,10,12 SPT(C) N=44		(5.10)				
9.70 9.70-10.15					11,13/12,12,14,16 SPT(C) N=54						

Remarks Borehole completed at 15.50m BGL. Borehole backfilled upon completion. Slotted pipe with pea gravel surround from 15.50m BGL to 1.0m BGL, plain pipe with bentonite seal from 1.0m BGL to ground level, finished with an upright cover.	Scale (approx)	Logged By
	1:50	CB
	Figure No. 9225-11-19.RC09	



Machine : Beretta T44 Flush : Water Core Dia: 63.5 mm Method : Rotary Cored	Casing Diameter 96mm cased to 15.50m	Ground Level (mOD) 25.01	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724967.4 E 759062.1 N	Dates 26/03/2020- 27/03/2020	Project Contractor Ground Investigations Ireland	Sheet 2/2

Depth (m)	TCR	SCR	RQD	FI	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
11.20 11.20-11.21	47	-			25/50 SPT(C) 25*/10 50/0	13.51	11.50	Very stiff pinkish brown sandy gravelly CLAY. Gravel is subangular-subrounded fine-coarse.			
12.70 12.70-12.71	47	-			22,3/50 SPT(C) 25*/10 50/0	(1.60)					
13.10	100	64	49			11.91	13.10	Medium strong light grey fine grained LIMESTONE partially-distinctly weathered with occasional calcite veining. One set of fractures. F1 20-30 degrees. Closely spaced undulating closed smooth.			
14.20	100	83	83	8		(2.40)					
15.50						9.51	15.50	Complete at 15.50m			

Remarks	Scale (approx)	Logged By
	1:50	CB
	Figure No. 9225-11-19.RC09	



Machine : Beretta t44 Flush : Water Core Dia: 63.5 mm Method : Rotary Cored	Casing Diameter 96mm cased to 18.20m	Ground Level (mOD) 24.03	Client DBFL	Job Number 9225-11-19
	Location (dGPS) 724951 E 759129.3 N	Dates 27/03/2020	Project Contractor Ground Investigations Ireland	Sheet 1/2

Depth (m)	TCR	SCR	RQD	FI	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.00	-	-				23.83	(0.20) 0.20	TOPSOIL Soft light brown slightly sandy gravelly CLAY. Gravel is fine to coarse subangular to subrounded.		
2.20 2.20-2.65	57	-			1,2/4,4,5,4 SPT(C) N=17	21.73	(2.10) 2.30	Stiff light brown slightly sandy gravelly CLAY. Gravel is fine to coarse subangular to subrounded.		
3.70 3.70-4.15	20	-			6,6/7,8,6,5 SPT(C) N=26	20.33	(1.40) 3.70	Poor recovery. Recovery consists of slightly sandy clayey GRAVEL with occasional cobbles of varying lithologies. Gravel is fine to coarse subangular to subrounded. Driller notes: fine Sand and gravelly Clay.		
5.20 5.20-5.65	20	-			7,6/6,7,8,7 SPT(C) N=28					
6.70 6.70-7.15	17	-			6,7/8,8,9,9 SPT(C) N=34					
8.20 8.20-8.65	20	-			7,7/9,8,8,9 SPT(C) N=34		(10.50)			
9.70 9.70-10.15					6,6/5,4,6,5 SPT(C) N=20					

Remarks Borehole completed at 18.20m BGL. Borehole backfilled upon completion.	Scale (approx)	Logged By
	1:50	CB
	Figure No. 9225-11-19.RC10	



Machine : Beretta t44		Casing Diameter 96mm cased to 18.20m		Ground Level (mOD) 24.03		Client DBFL		Job Number 9225-11-19	
Flush : Water		Location (dGPS) 724951 E 759129.3 N		Dates 27/03/2020		Project Contractor Ground Investigations Ireland		Sheet 2/2	
Core Dia: 63.5 mm									
Method : Rotary Cored									

Depth (m)	TCR	SCR	RQD	FI	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
11.20 11.20-11.65	23	-			9,10/10,9,8,10 SPT(C) N=37					
12.70 12.70-13.15	23	-			11,12/11,10,12,10 SPT(C) N=43					
14.20 14.20-14.20	10	-			25/50 SPT(C) 25*0 50/0	9.83	14.20 (0.40)	Very stiff dark brown slightly sandy slightly gravelly CLAY. Gravel is fine to coarse subangular to subrounded.		
	80	-				9.43	14.60	Possible weathered rock. Hard pinkish grey gravelly CLAY. Gravel is fine to coarse subangular to subrounded.		
15.70 15.70-15.70	33	-			25/50 SPT(C) 25*0 50/0		(2.60)			
17.20						6.83	17.20	Complete at 17.20m		

Remarks	Scale (approx)	Logged By
	1:50	CB
	Figure No. 9225-11-19.RC10	

Hackettstown - Rotary Core Photographs

BH02



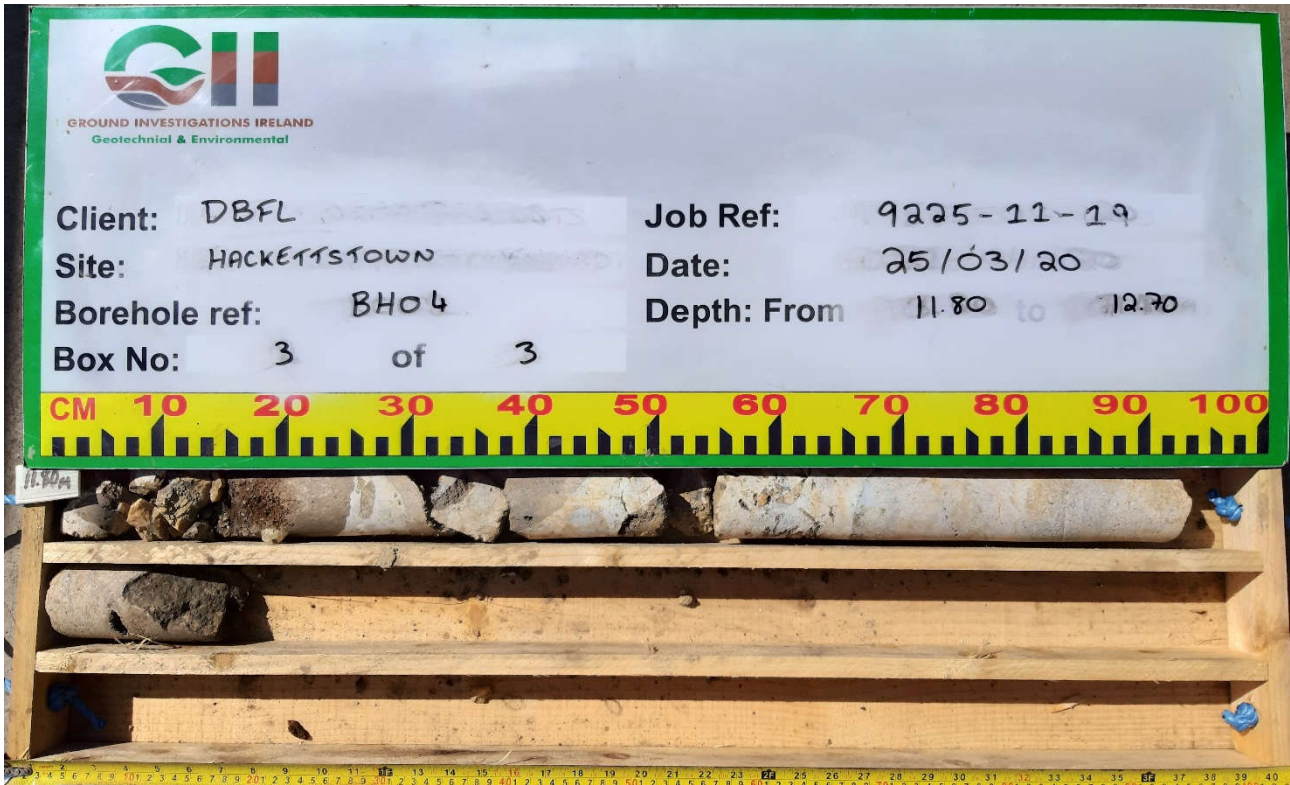
Hackettstown - Rotary Core Photographs

BH04



Hackettstown - Rotary Core Photographs

BH04



Hackettstown - Rotary Core Photographs

RC08



Hackettstown - Rotary Core Photographs



GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

Client: DBFL Job ref: 9225-11-19
Site: HACKETTSTOWN, SKERRIES Date: 26/03/20
Borehole ref: RCO8 Depth: From 9.70m to 12.70m
Box No: 3 of 3



Hackettstown - Rotary Core Photographs

RC09



GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

Client: DBFL Job Ref: 9225-11-19
Site: HACKETTSTOWN, SKERRIES Date: 26/03/20
Borehole ref: RC09 Depth: From 0.0m to 3.20m
Box No: 1 of 4



GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

Client: DBFL Job Ref: 9225-11-19
Site: HACKETTSTOWN, SKERRIES Date: 26/03/20
Borehole ref: RC09 Depth: From 3.20m to 8.20m
Box No: 2 of 4



Hackettstown - Rotary Core Photographs



GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

Client: DBFL Job Ref: 9225-11-19
Site: HACKETTSTOWN, SKERRIES Date: 26/03/20
Borehole ref: RC09 Depth: From 8.20m to 12.70m
Box No: 3 of 4

CM 10 20 30 40 50 60 70 80 90 100



GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

Client: DBFL Job Ref: 9225-11-19
Site: HACKETTSTOWN, SKERRIES Date: 26/03/20
Borehole ref: RC09 Depth: From 12.70m to 15.50m
Box No: 4 of 4

CM 10 20 30 40 50 60 70 80 90 100



Hackettstown - Rotary Core Photographs

RC10



GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

Client: DBFL

Job Ref: 9225-11-19

Site: HACKETTSTOWN, SKERRIES

Date: 27/03/20

Borehole ref: Rc104

Depth: From 0.0m to 3.70m

Box No: 1 of 3

CM 10 20 30 40 50 60 70 80 90 100



GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

Client: DBFL

Job Ref: 9225-11-19

Site: HACKETTSTOWN, SKERRIES

Date: 27/03/20

Borehole ref: Rc104

Depth: From 3.70m to 14.50m

Box No: 2 of 3

CM 10 20 30 40 50 60 70 80 90 100



Hackettstown - Rotary Core Photographs



GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

Client: DBFL

Job Ref: 9225-11-19

Site: HACKETTSTOWN, SKERRIES

Date: 27/03/20

Borehole ref: Rc10

Depth: From 14.50m to 17.20m

Box No: 3 of 3



APPENDIX 6 – Laboratory Testing



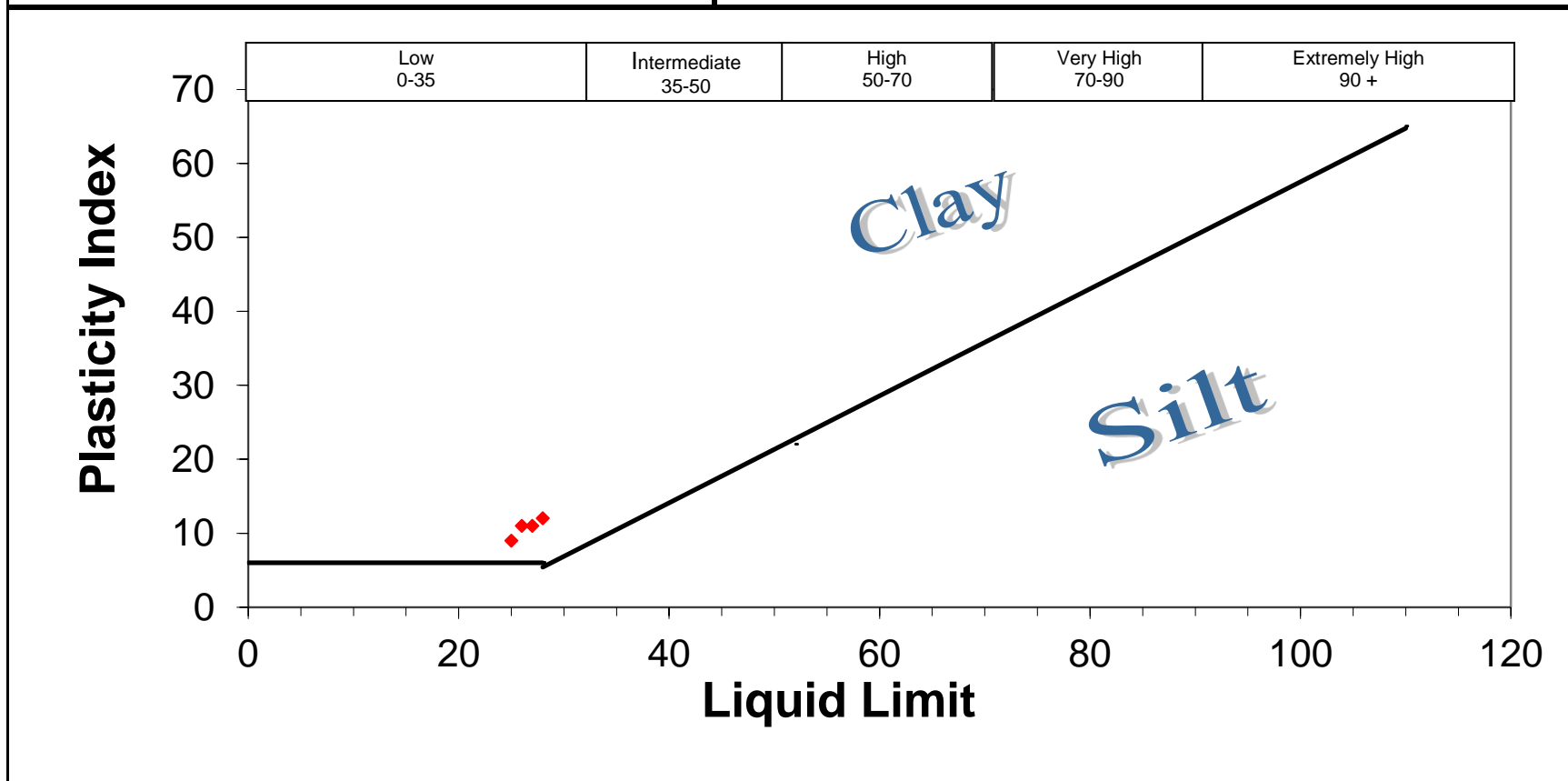
National Materials Testing Laboratory Ltd.

SUMMARY OF TEST RESULTS

BH/TP No	Depth m	Sample No.	Moisture %	Particle		Index Properties			Bulk	Cell	Undrained Triaxial Tests		Lab	Remarks	
				Density Mg/m3	<425um %	LL %	PL %	PI %	Density Mg/m3	Presssure kPa	Compressive Stress kPa	Strain at Failure %	Vane kPa		
TP01	2.70	B	15.5		73.3	28	16	12							
TP09	1.50	B	13.3		52.5	25	16	9							
TP10	1.50	B	19.7		52.5	26	15	11							
TP103	1.50	B	14.1		69.5	27	16	11							
NMTL		Notes : 1. All BS tests carried out using preferred (definitive) method unless otherwise stated.									Job ref No.	NMTL 3139	GII Project ID:	9225-11-19	
											Location	Hackettstown			

NMTL LTD
Unit 18c, Tullow Industrial Estate
Tullow
County Carlow
Tel: 00353 59 9180822
Mob: 00353 872575508
billa@nmtl.ie

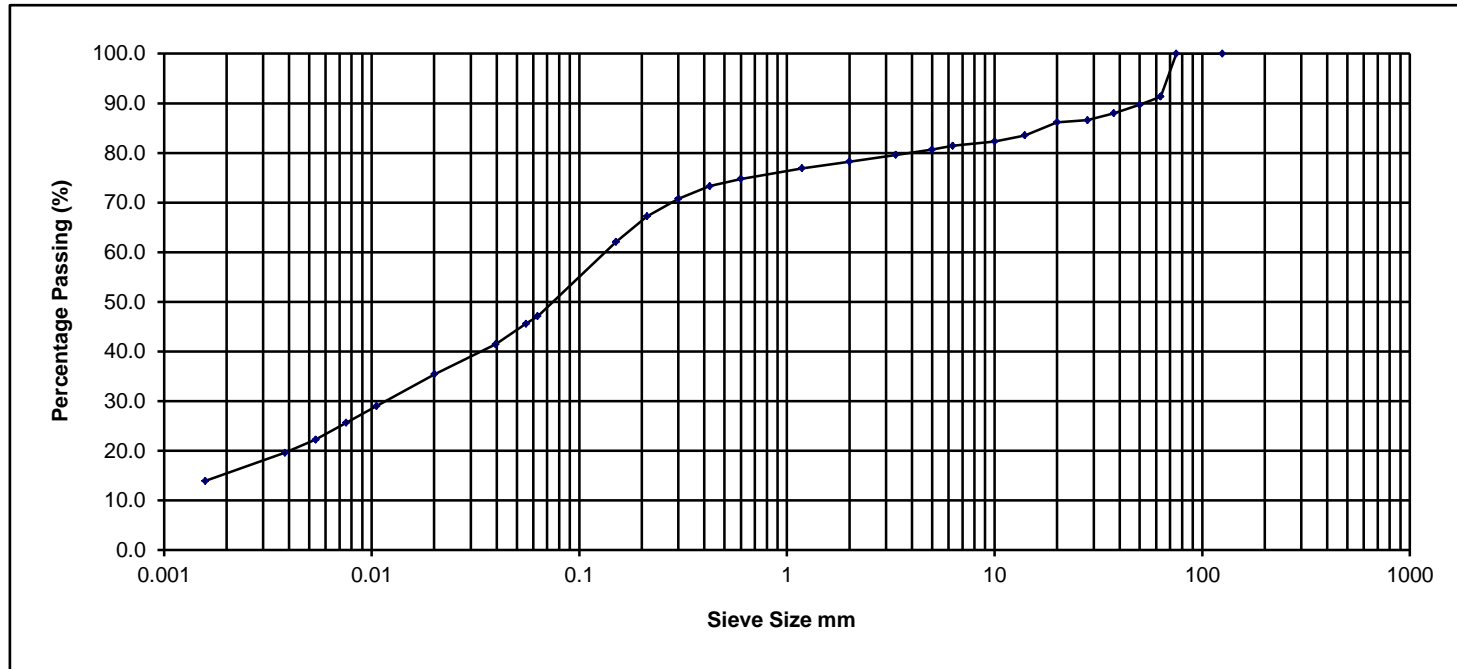
Contract: Hackettstown
Client: Ground Investigations Ireland Ltd
Engineer: Mike Sutton
GII Project ID 9225-11-19
Date: 11/02/2020
Tested By: Sb/Tch/Ms **Checked:** Bc
Job ref No. NMTL 3139



NMTL Ltd

Sieve Size mm	% Passing
125.000	100.0
75.000	100.0
63.000	91.3
50.000	89.7
37.500	88.0
28.000	86.6
20.000	86.2
14.000	83.5
10.000	82.3
6.300	81.4
5.000	80.7
3.350	79.6
2.000	78.3
1.180	76.9
0.600	74.8
0.425	73.3
0.300	70.8
0.212	67.2
0.150	62.1
0.063	47.1
0.055	45.6
0.040	41.5
0.020	35.4
0.011	29.0
0.008	25.6
0.005	22.2
0.004	19.6
0.002	13.9

Determination of Particle Size Distribution BS 1377 : 1990 : Part 2 : Clauses 9.2 & 9.5



Clay	Percentage Particle Size						Cobbles	Boulder
	Fine	Medium	Coarse	Fine	Medium	Coarse		
	Silt		Sand		Gravel			
13.9	33.2		31.1		13.1		8.7	0.0

Sample Description: Brown slightly gravelly slightly sandy silty CLAY.

Project No. NMTL 3139

BH/TP No. TP01

Project Hackettstown

GII PROJECT ID: 9225-11-19

Sample No. B

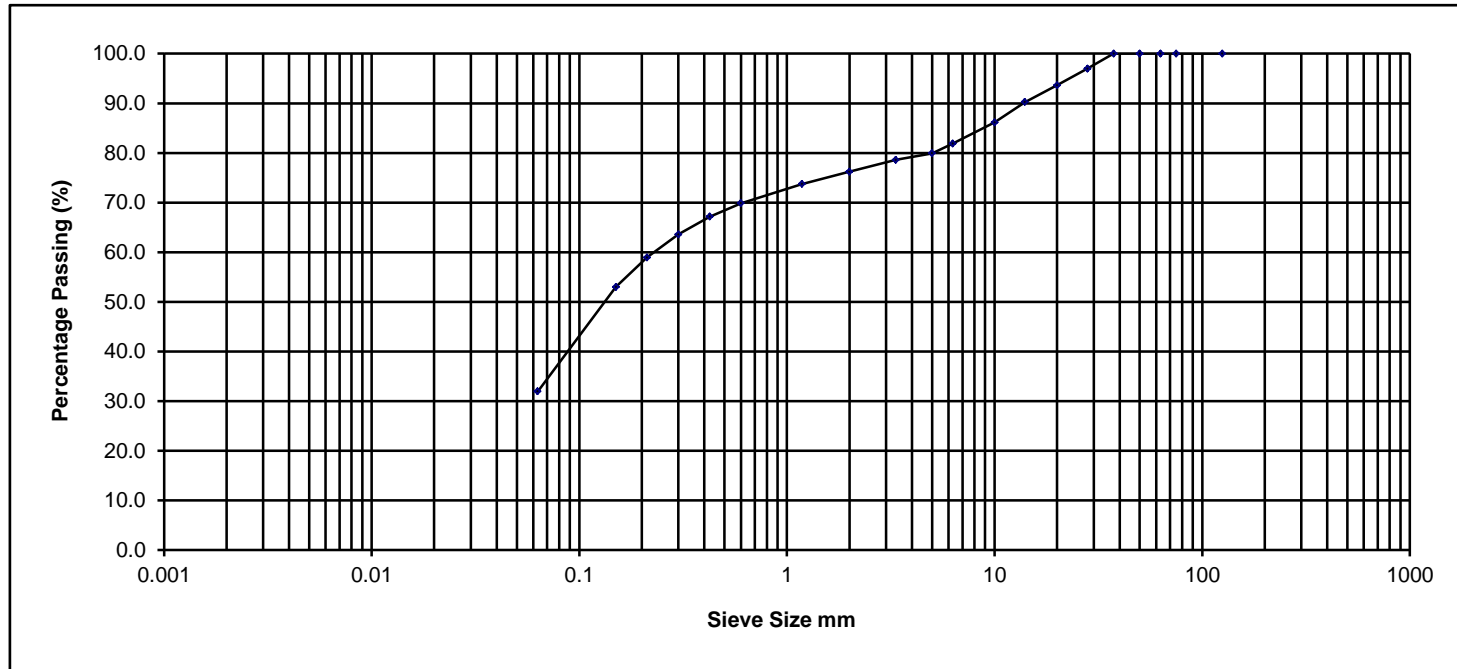
NM
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Ltd

Operator	Tch	Checked	Nc	Approved	Bc	Date sample tested	07/02/2020	Depth	2.70m
----------	-----	---------	----	----------	----	--------------------	------------	-------	-------

NMTL Ltd

Sieve Size mm	% Passing
125.000	100.0
75.000	100.0
63.000	100.0
50.000	100.0
37.500	100.0
28.000	97.0
20.000	93.6
14.000	90.2
10.000	86.1
6.300	81.9
5.000	79.9
3.350	78.6
2.000	76.2
1.180	73.7
0.600	69.9
0.425	67.1
0.300	63.6
0.212	58.9
0.150	53.0
0.063	31.9

Determination of Particle Size Distribution BS 1377 : 1990 : Part 2 : Clauses 9.2 & 9.5



Percentage Particle Size

Clay	Fine			Medium			Coarse			Cobbles	Boulder
	Fine	Medium	Coarse	Fine	Medium	Coarse	Fine	Medium	Coarse		
	Silt			Sand			Gravel				
	31.9			44.3			23.8			0.0	0.0

Sample Description Brown slightly gravelly sandy SILT/CLAY

Project No.

NMTL 3139

BH/TP No.

TP05

Project

0

GII PROJECT ID: 9225-11-19

Sample No.

B

NM

TL

Ltd

Operator

Tch

Checked

Nc

Approved

Bc

Date sample tested

07/02/2020

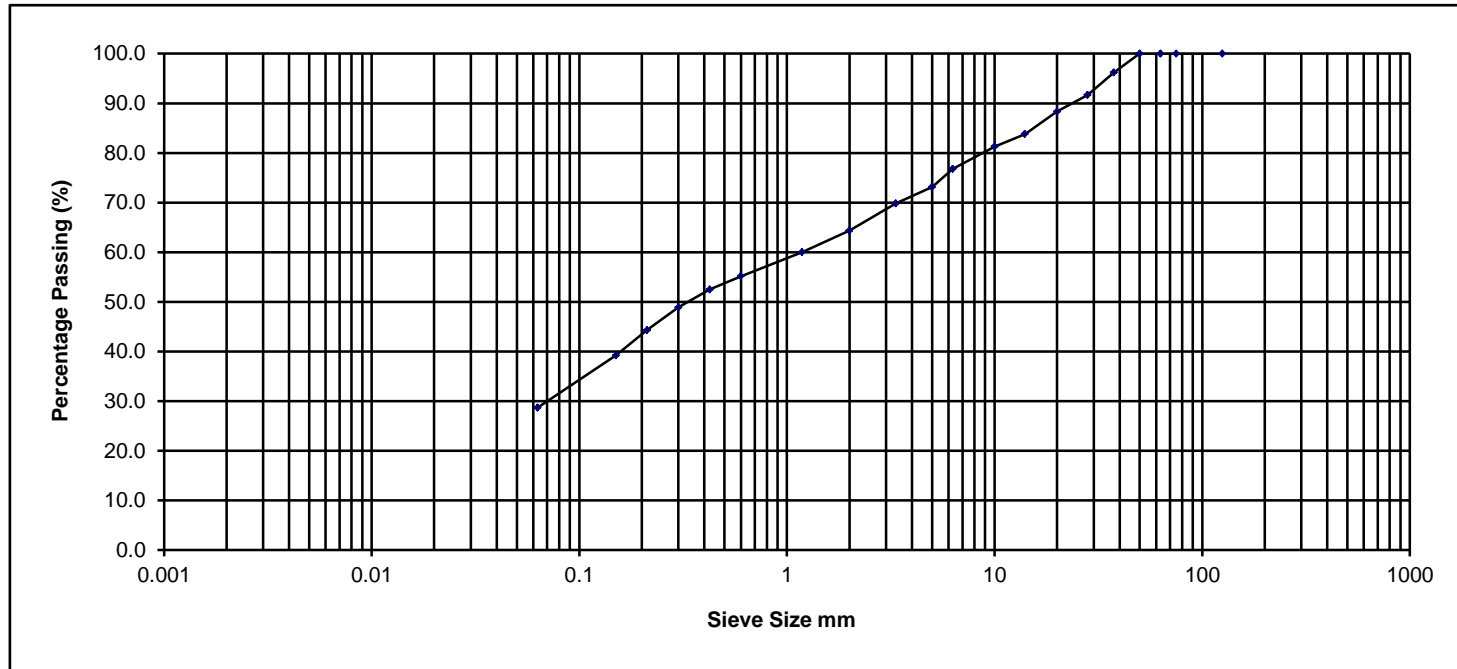
Depth

2.50m

NMTL Ltd

Sieve Size mm	% Passing
125.000	100.0
75.000	100.0
63.000	100.0
50.000	100.0
37.500	96.2
28.000	91.7
20.000	88.3
14.000	83.8
10.000	81.3
6.300	76.8
5.000	73.1
3.350	69.8
2.000	64.3
1.180	60.0
0.600	55.2
0.425	52.5
0.300	48.9
0.212	44.3
0.150	39.2
0.063	28.7

Determination of Particle Size Distribution BS 1377 : 1990 : Part 2 : Clauses 9.2 & 9.5



Percentage Particle Size

Clay	Fine			Medium			Coarse			Cobbles	Boulder
	Fine	Medium	Coarse	Fine	Medium	Coarse	Fine	Medium	Coarse		
	Silt			Sand			Gravel				
	28.7			35.6			35.7			0.0	0.0

Sample Description Brown gravelly sandy silty CLAY

Project No.

NMTL 3139

BH/TP No.

TP09

Project

0

GII PROJECT ID: 9225-11-19

Sample No.

B

NM

TL

Ltd

Operator Tch

Checked Nc

Approved Bc

Date sample tested

07/02/2020

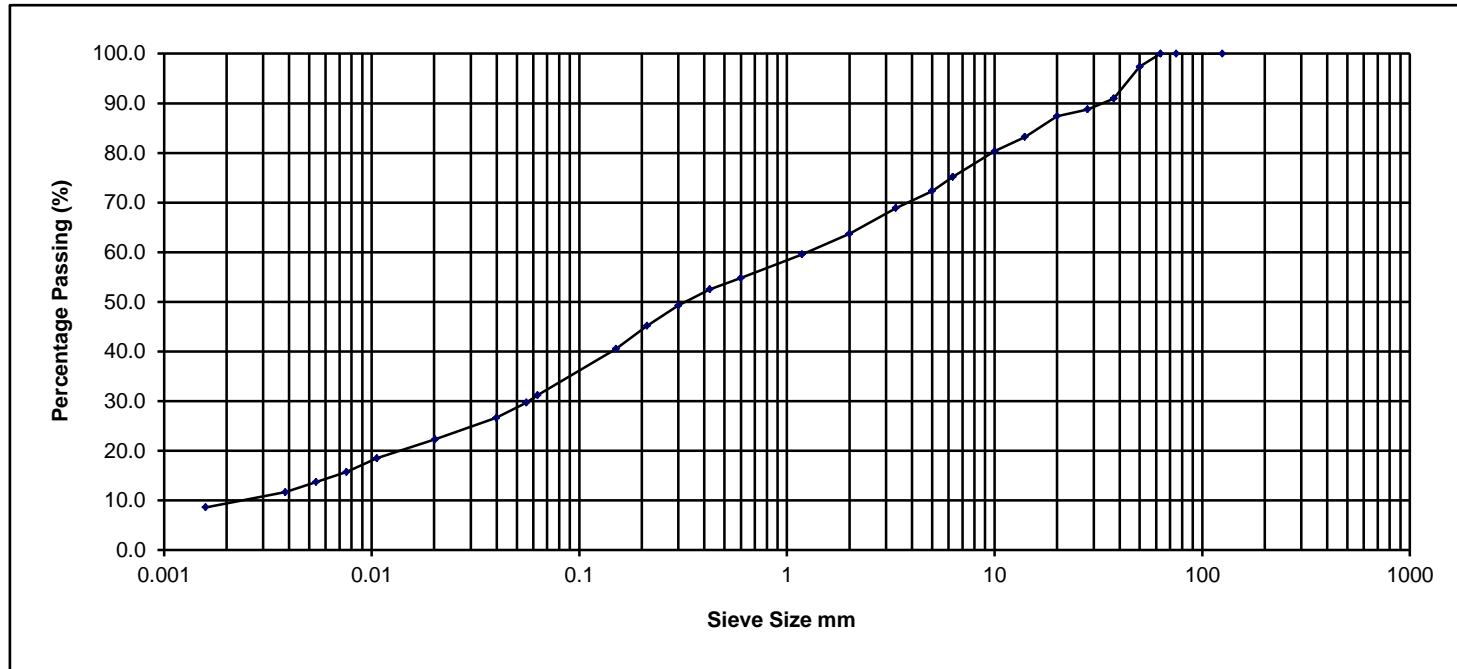
Depth

1.5m

NMTL Ltd

Sieve Size mm	% Passing
125.000	100.0
75.000	100.0
63.000	100.0
50.000	97.4
37.500	90.9
28.000	88.8
20.000	87.4
14.000	83.2
10.000	80.3
6.300	75.2
5.000	72.3
3.350	68.9
2.000	63.7
1.180	59.6
0.600	54.8
0.425	52.5
0.300	49.3
0.212	45.2
0.150	40.5
0.063	31.2
0.056	29.7
0.040	26.6
0.020	22.3
0.011	18.5
0.008	15.7
0.005	13.7
0.004	11.7
0.002	8.6

Determination of Particle Size Distribution BS 1377 : 1990 : Part 2 : Clauses 9.2 & 9.5



Clay	Percentage Particle Size						Cobbles	Boulder
	Fine	Medium	Coarse	Fine	Medium	Coarse		
	Silt		Sand		Gravel			
8.6	22.6		32.5		36.3		0.0	0.0

Sample Description: Brown slightly sandy gravelly silty CLAY.

Project No. NMTL 3139

BH/TP No. TP10

Project Hackettstown

GII PROJECT ID: 9225-11-19

Sample No. B

NMTL Ltd

Operator	Tch	Checked	Nc	Approved	Bc	Date sample tested	07/02/2020	Depth	1.50m
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DETERMINATION OF THE CALIFORNIA BEARING RATIO TEST
BS 1377 : PART 4 : CLAUSE 7 : 1990

Soil Description Light brown slightly sandy slightly gravelly SILT/CLAY Date 13-Dec-19

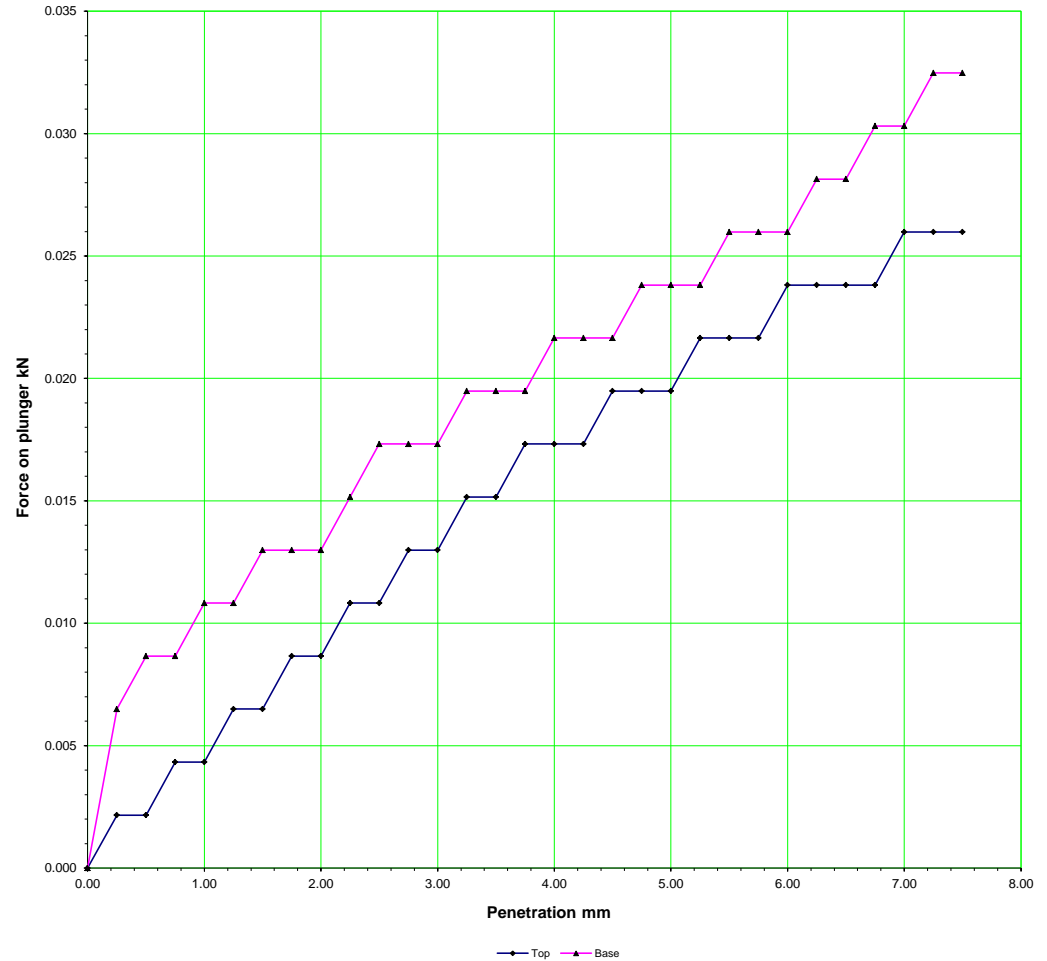
Test Method BS 1377: Part 4 : 1990 :7.4 Force Measuring Device VJT 08211 Test 1

Preparatic Remoulded with 2.5 kg rammer at natural moisture content Surcharge 10 kPa Mean Calibration 4.33 N/Div

Penetration Force Gauge Force on 4.33 N/Div of plunger reading divisions plunger kN California Bearing Ratio Results %

mm	Top	Bottom	Top	Bottom	Top	Base
0.00	0.0	0.0	0.000	0.000		
0.25	0.5	1.5	0.002	0.006		
0.50	0.5	2.0	0.002	0.009		
0.75	1.0	2.0	0.004	0.009		
1.00	1.0	2.5	0.004	0.011		
1.25	1.5	2.5	0.006	0.011		
1.50	1.5	3.0	0.006	0.013		
1.75	2.0	3.0	0.009	0.013		
2.00	2.0	3.0	0.009	0.013		
2.25	2.5	3.5	0.011	0.015		
2.50	2.5	4.0	0.011	0.017	0.08	0.13
2.75	3.0	4.0	0.013	0.017		
3.00	3.0	4.0	0.013	0.017		
3.25	3.5	4.5	0.015	0.019		
3.50	3.5	4.5	0.015	0.019		
3.75	4.0	4.5	0.017	0.019		
4.00	4.0	5.0	0.017	0.022		
4.25	4.0	5.0	0.017	0.022		
4.50	4.5	5.0	0.019	0.022		
4.75	4.5	5.5	0.019	0.024		
5.00	5.0	5.5	0.019	0.024	0.10	0.12
5.25	5.0	5.5	0.022	0.024		
5.50	5.0	6.0	0.022	0.026		
5.75	5.0	6.0	0.022	0.026		
6.00	5.5	6.0	0.024	0.026		
6.25	5.5	6.5	0.024	0.028		
6.50	5.5	6.5	0.024	0.028		
6.75	5.5	7.0	0.024	0.030		
7.00	6.0	7.0	0.026	0.030		
7.25	6.0	7.5	0.026	0.032		
7.50	6.0	7.5	0.026	0.032		

Moisture content after test		Top	Middle	Base	Specimen wt g	4585
Container No.		Tray	Tray	Tray	Diameter mm	152
Mass of wet soil + container	g	1842.9	1812.9	1390.2	Length mm	127.0
Mass of dry soil + container	g	1546.8	1525.4	1170.0		
Weight of container	g	146.2	186.1	142.8		
Mass of moisture	g	296.1	287.5	220.2	Average MC %	21.35
Dry weight	g	1400.6	1339.3	1027.2	Density Mg/m3	1.99
Moisture content	%	21.1	21.5	21.4	Dry Density Mg/m3	1.64



NM
TL
Ltd

Project: Hackettstown

GII Project ID: 9225-11-19

	Date	Project No.	NMTL3107
Operator	Tch 13-Dec-19	Trial Pit No.	CBR19
Checked	Nc	Sample No.	B
Approved	Bc	Depth	0.50m

DETERMINATION OF THE CALIFORNIA BEARING RATIO TEST
BS 1377 : PART 4 : CLAUSE 7 : 1990

Soil Description Brown slightly sandy slightly gravelly SILT/CLAY

Date 13-Dec-19

Test Method BS 1377: Part 4 : 1990 :7.4

Force Measuring Device VJT 08211

Test 1

Preparatic Remoulded with 2.5 kg rammer at natural moisture content

Surcharge 10 kPa

Mean Calibration 4.33

N/Div

Penetration Force Gauge

Force on plunger 4.33

N/Div

of plunger mm

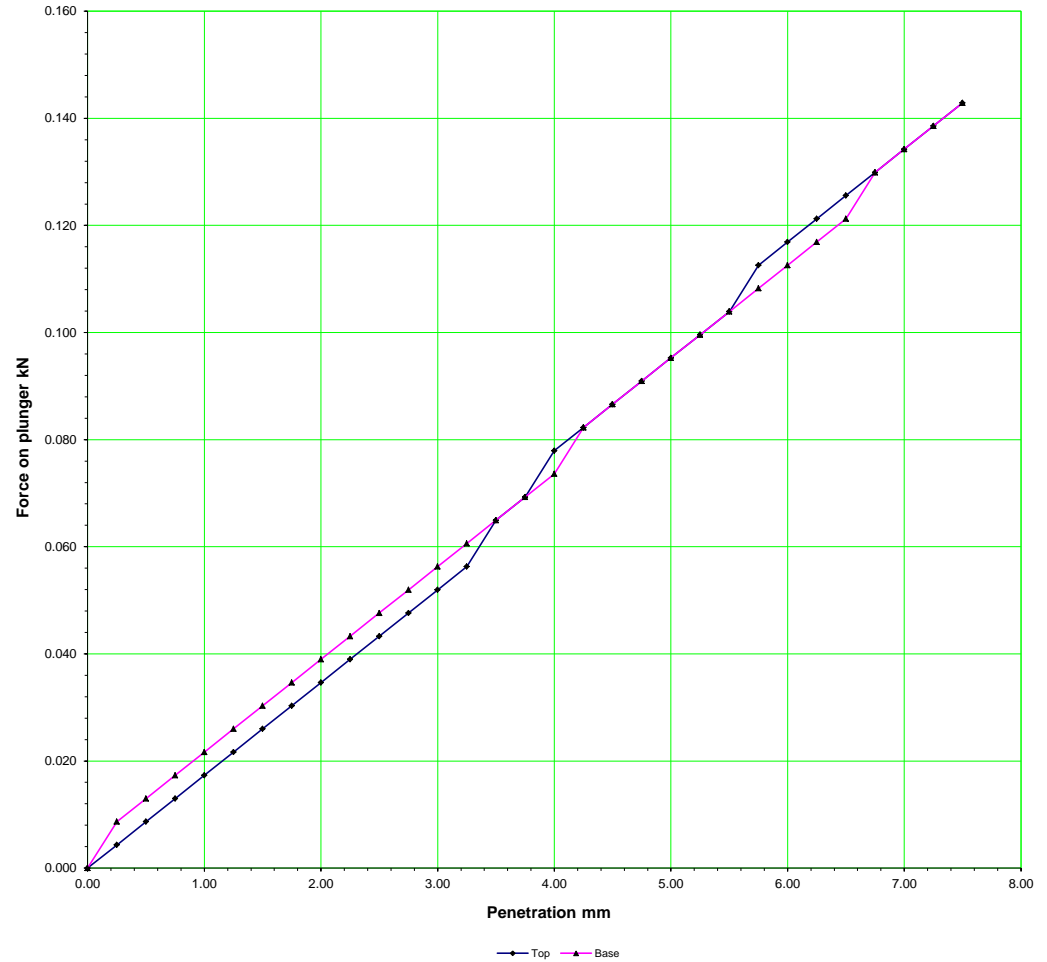
reading divisions

plunger kN

California Bearing Ratio Results %

	Top	Bottom	Top	Bottom	Top	Base
0.00	0.0	0.0	0.000	0.000		
0.25	1.0	2.0	0.004	0.009		
0.50	2.0	3.0	0.009	0.013		
0.75	3.0	4.0	0.013	0.017		
1.00	4.0	5.0	0.017	0.022		
1.25	5.0	6.0	0.022	0.026		
1.50	6.0	7.0	0.026	0.030		
1.75	7.0	8.0	0.030	0.035		
2.00	8.0	9.0	0.035	0.039		
2.25	9.0	10.0	0.039	0.043		
2.50	10.0	11.0	0.043	0.048	0.33	0.36
2.75	11.0	12.0	0.048	0.052		
3.00	12.0	13.0	0.052	0.056		
3.25	13.0	14.0	0.056	0.061		
3.50	15.0	15.0	0.065	0.065		
3.75	16.0	16.0	0.069	0.069		
4.00	18.0	17.0	0.078	0.074		
4.25	19.0	19.0	0.082	0.082		
4.50	20.0	20.0	0.087	0.087		
4.75	21.0	21.0	0.091	0.091		
5.00	22.0	22.0	0.095	0.095	0.48	0.48
5.25	23.0	23.0	0.100	0.100		
5.50	24.0	24.0	0.104	0.104		
5.75	26.0	25.0	0.113	0.108		
6.00	27.0	26.0	0.117	0.113		
6.25	28.0	27.0	0.121	0.117		
6.50	29.0	28.0	0.126	0.121		
6.75	30.0	30.0	0.130	0.130		
7.00	31.0	31.0	0.134	0.134		
7.25	32.0	32.0	0.139	0.139		
7.50	33.0	33.0	0.143	0.143		

Moisture content after test		Top	Middle	Base	Specimen wt g	4685
Container No.		Tray	Tray	Tray	Diameter mm	152
Mass of wet soil + container	g	1759.2	1727.7	1671.1	Length mm	127.0
Mass of dry soil + container	g	1516.8	1482.4	1436.4		
Weight of container	g	190.7	145.3	148.8		
Mass of moisture	g	242.4	245.4	234.7	Average MC %	18.29
Dry weight	g	1326.1	1337.0	1287.6	Density Mg/m3	2.03
Moisture content	%	18.3	18.4	18.2	Dry Density Mg/m3	1.72



NM
TL
Ltd

Project: Hackettstown

GII Project ID: 9225-11-19

	Date	Project No.	NMTL3107
Operator	Tch 13-Dec-19	Trial Pit No.	CBR20
Checked	Nc	Sample No.	B
Approved	Bc	Depth	0.50m

DETERMINATION OF THE CALIFORNIA BEARING RATIO TEST
BS 1377 : PART 4 : CLAUSE 7 : 1990

Soil Description Light brown/orange brown slightly sandy slightly gravelly SILT/CLAY
Date 13-Dec-19

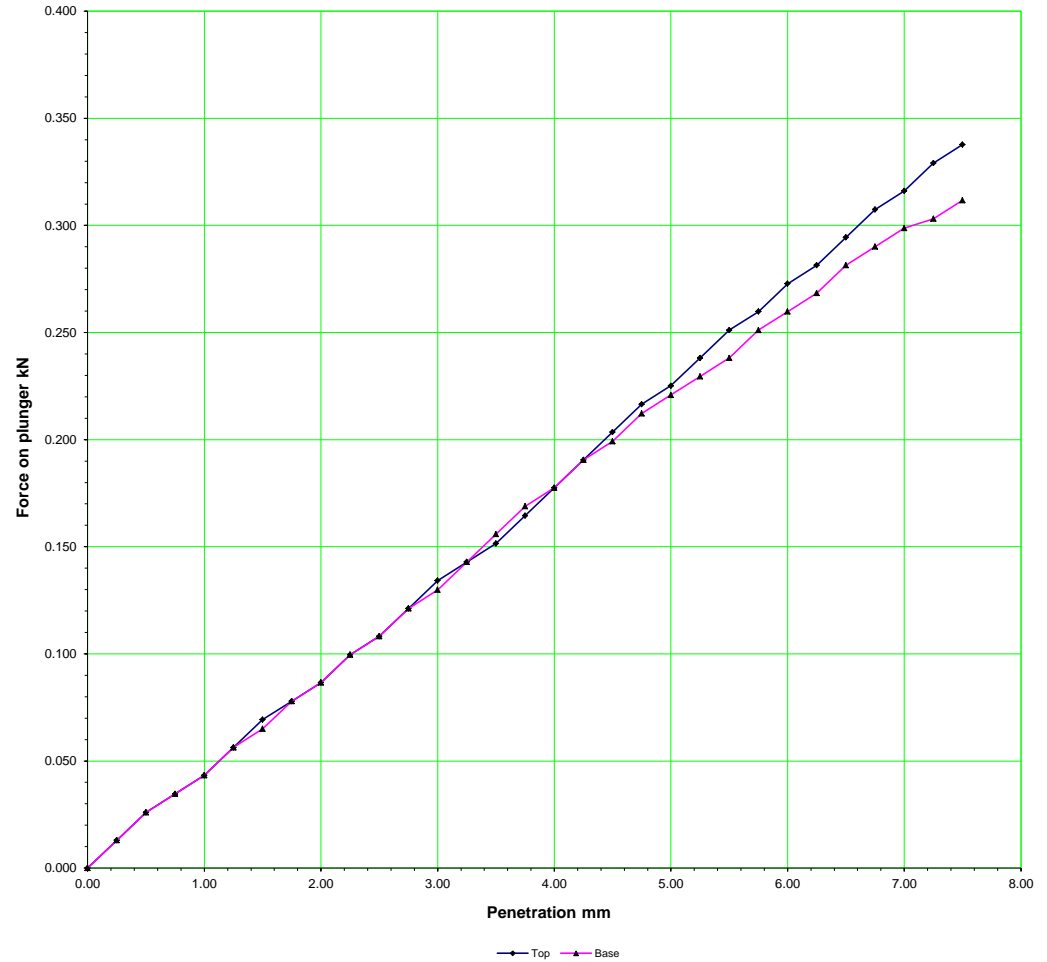
Test Method BS 1377: Part 4 : 1990 :7.4
Force Measuring Device VJT 08211
Test 1

Preparatic Remoulded with 2.5 kg rammer at natural moisture content
Surcharge 10 kPa
Mean Calibration 4.33 N/Div

Penetration Force Gauge
Force on 4.33 N/Div
California Bearing Ratio Results

mm	reading divisions		plunger kN		California Bearing Ratio Results %	
	Top	Bottom	Top	Bottom	Top	Base
0.00	0.0	0.0	0.000	0.000		
0.25	3.0	3.0	0.013	0.013		
0.50	6.0	6.0	0.026	0.026		
0.75	8.0	8.0	0.035	0.035		
1.00	10.0	10.0	0.043	0.043		
1.25	13.0	13.0	0.056	0.056		
1.50	16.0	15.0	0.069	0.065		
1.75	18.0	18.0	0.078	0.078		
2.00	20.0	20.0	0.087	0.087		
2.25	23.0	23.0	0.100	0.100		
2.50	25.0	25.0	0.108	0.108	0.82	0.82
2.75	28.0	28.0	0.121	0.121		
3.00	31.0	30.0	0.134	0.130		
3.25	33.0	33.0	0.143	0.143		
3.50	35.0	36.0	0.152	0.156		
3.75	38.0	39.0	0.165	0.169		
4.00	41.0	41.0	0.178	0.178		
4.25	44.0	44.0	0.191	0.191		
4.50	47.0	46.0	0.204	0.199		
4.75	50.0	49.0	0.217	0.212		
5.00	52.0	51.0	0.225	0.221	1.13	1.10
5.25	55.0	53.0	0.238	0.229		
5.50	58.0	55.0	0.251	0.238		
5.75	60.0	58.0	0.260	0.251		
6.00	63.0	60.0	0.273	0.260		
6.25	65.0	62.0	0.281	0.268		
6.50	68.0	65.0	0.294	0.281		
6.75	71.0	67.0	0.307	0.290		
7.00	73.0	69.0	0.316	0.299		
7.25	76.0	70.0	0.329	0.303		
7.50	78.0	72.0	0.338	0.312		

	Top	Middle	Base	Specimen wt g	4455
Moisture content after test	Tray	Tray	Tray	Diameter mm	152
Container No.				Length mm	127.0
Mass of wet soil + container	g	1610.6	1698.0	1599.2	
Mass of dry soil + container	g	1330.8	1399.3	1326.3	
Weight of container	g	148.9	144.9	188.2	
Mass of moisture	g	279.8	298.7	273.0	Average MC %
Dry weight	g	1181.9	1254.3	1138.1	Density Mg/m3
Moisture content	%	23.7	23.8	24.0	Dry Density Mg/m3
					1.56



NM
TL
Ltd

Project: Hackettstown

GII Project ID: 9225-11-19

		Date	Project No.	NMTL3107
Operator	Tch	13-Dec-19	Trial Pit No.	CBR21
Checked	Nc		Sample No.	B
Approved	Bc		Depth	0.50m

DETERMINATION OF THE CALIFORNIA BEARING RATIO TEST
BS 1377 : PART 4 : CLAUSE 7 : 1990

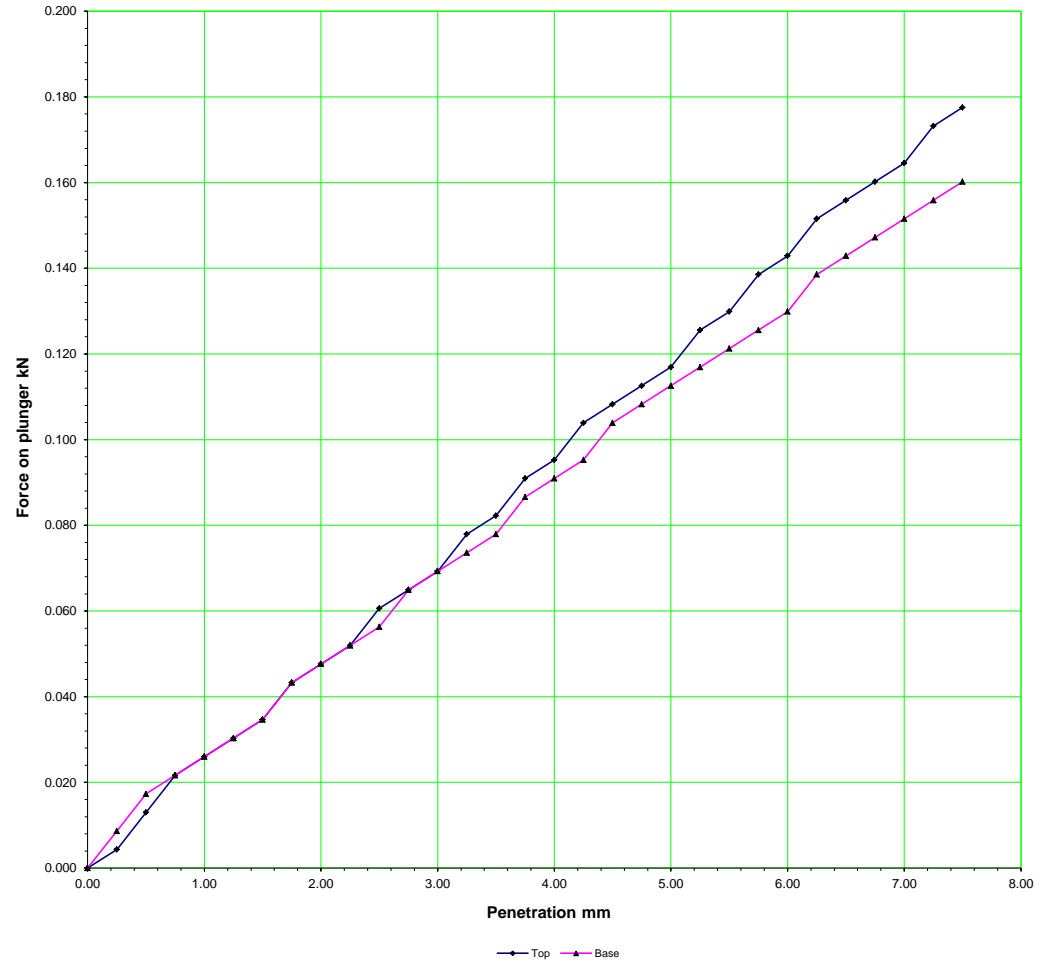
Soil Description Brown slightly sandy slightly gravelly clayey/SILT Date 16-Dec-19

Test Method BS 1377: Part 4 : 1990 :7.4 Force Measuring Device VJT 08211 Test 1

Preparatic Remoulded with 2.5 kg rammer at natural moisture content
Surcharge 10 kPa Mean Calibration 4.33 N/Div
Penetration Force Gauge Force on 4.33 N/Div
of plunger reading divisions plunger kN California Bearing Ratio Results %

	Top	Bottom	Top	Bottom	Top	Base
0.00	0.0	0.0	0.000	0.000		
0.25	1.0	2.0	0.004	0.009		
0.50	3.0	4.0	0.013	0.017		
0.75	5.0	5.0	0.022	0.022		
1.00	6.0	6.0	0.026	0.026		
1.25	7.0	7.0	0.030	0.030		
1.50	8.0	8.0	0.035	0.035		
1.75	10.0	10.0	0.043	0.043		
2.00	11.0	11.0	0.048	0.048		
2.25	12.0	12.0	0.052	0.052		
2.50	14.0	13.0	0.061	0.056	0.46	0.43
2.75	15.0	15.0	0.065	0.065		
3.00	16.0	16.0	0.069	0.069		
3.25	18.0	17.0	0.078	0.074		
3.50	19.0	18.0	0.082	0.078		
3.75	21.0	20.0	0.091	0.087		
4.00	22.0	21.0	0.095	0.091		
4.25	24.0	22.0	0.104	0.095		
4.50	25.0	24.0	0.108	0.104		
4.75	26.0	25.0	0.113	0.108		
5.00	27.0	26.0	0.117	0.113	0.58	0.56
5.25	29.0	27.0	0.126	0.117		
5.50	30.0	28.0	0.130	0.121		
5.75	32.0	29.0	0.139	0.126		
6.00	33.0	30.0	0.143	0.130		
6.25	35.0	32.0	0.152	0.139		
6.50	36.0	33.0	0.156	0.143		
6.75	37.0	34.0	0.160	0.147		
7.00	38.0	35.0	0.165	0.152		
7.25	40.0	36.0	0.173	0.156		
7.50	41.0	37.0	0.178	0.160		

Moisture content after test		Top	Middle	Base	Specimen wt g	4650
Container No.		Tray	Tray	Tray	Diameter mm	152
Mass of wet soil + container	g	1817.9	1858.7	1445.2	Length mm	127.0
Mass of dry soil + container	g	1554.6	1580.3	1236.3		
Weight of container	g	185.4	144.3	144.2		
Mass of moisture	g	263.3	278.3	208.9	Average MC %	19.25
Dry weight	g	1369.2	1436.1	1092.1	Density Mg/m3	2.02
Moisture content	%	19.2	19.4	19.1	Dry Density Mg/m3	1.69



NM
TL
Ltd

Project: Hackettstown

GII Project ID: 9225-11-19

	Date	Project No.	NMTL3107
Operator	Tch 16-Dec-19	Trial Pit No.	CBR22
Checked	Nc	Sample No.	B
Approved	Bc	Depth	0.50m

DETERMINATION OF THE CALIFORNIA BEARING RATIO TEST
BS 1377 : PART 4 : CLAUSE 7 : 1990

Soil Description Brown slightly gravelly silty SAND

Date 16-Dec-19

Test Method BS 1377: Part 4 : 1990 :7.4

Force Measuring Device VJT 08211

Test 1

Preparatc Remoulded with 2.5 kg rammer at natural moisture content

Surcharge 10 kPa

Mean Calibration 4.33

N/Div

Penetration Force Gauge

Force on 4.33

N/Div

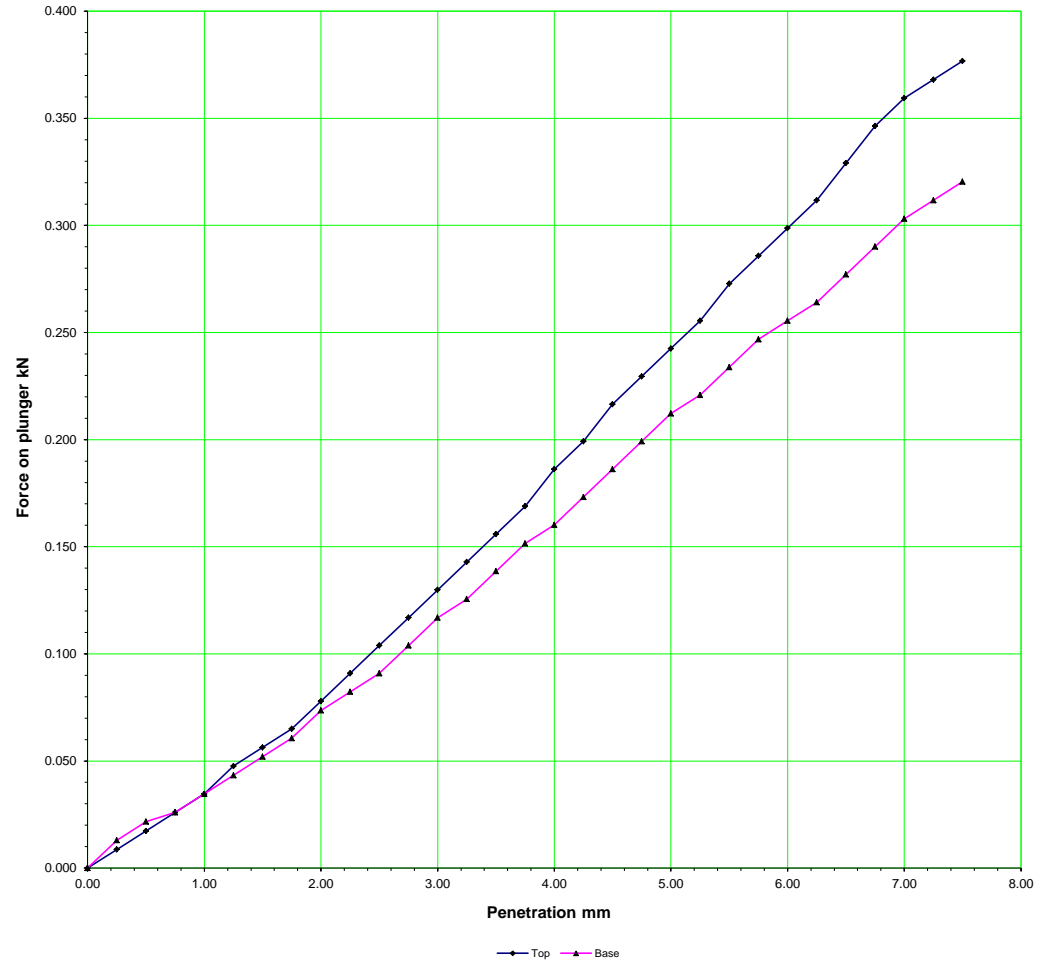
of plunger reading divisions

plunger kN

California Bearing Ratio Results %

mm	Top	Bottom	Top	Bottom	Top	Base
0.00	0.0	0.0	0.000	0.000		
0.25	2.0	3.0	0.009	0.013		
0.50	4.0	5.0	0.017	0.022		
0.75	6.0	6.0	0.026	0.026		
1.00	8.0	8.0	0.035	0.035		
1.25	11.0	10.0	0.048	0.043		
1.50	13.0	12.0	0.056	0.052		
1.75	15.0	14.0	0.065	0.061		
2.00	18.0	17.0	0.078	0.074		
2.25	21.0	19.0	0.091	0.082		
2.50	24.0	21.0	0.104	0.091	0.79	0.69
2.75	27.0	24.0	0.117	0.104		
3.00	30.0	27.0	0.130	0.117		
3.25	33.0	29.0	0.143	0.126		
3.50	36.0	32.0	0.156	0.139		
3.75	39.0	35.0	0.169	0.152		
4.00	43.0	37.0	0.186	0.160		
4.25	46.0	40.0	0.199	0.173		
4.50	50.0	43.0	0.217	0.186		
4.75	53.0	46.0	0.229	0.199		
5.00	56.0	49.0	0.242	0.212	1.21	1.06
5.25	59.0	51.0	0.255	0.221		
5.50	63.0	54.0	0.273	0.234		
5.75	66.0	57.0	0.286	0.247		
6.00	69.0	59.0	0.299	0.255		
6.25	72.0	61.0	0.312	0.264		
6.50	76.0	64.0	0.329	0.277		
6.75	80.0	67.0	0.346	0.290		
7.00	83.0	70.0	0.359	0.303		
7.25	85.0	72.0	0.368	0.312		
7.50	87.0	74.0	0.377	0.320		

	Top	Middle	Base	Specimen wt g
Moisture content after test	Tray	Tray	Tray	5020
Container No.				Diameter mm 152
Mass of wet soil + container	g 2138.4	g 1935.1	g 1496.9	Length mm 127.0
Mass of dry soil + container	g 1908.1	g 1726.2	g 1339.4	
Weight of container	g 189.2	g 187.3	g 188.7	
Mass of moisture	g 230.3	g 208.9	g 157.5	Average MC % 13.55
Dry weight	g 1718.9	g 1538.9	g 1150.7	Density Mg/m3 2.18
Moisture content	% 13.4	% 13.6	% 13.7	Dry Density Mg/m3 1.92



NM
TL
Ltd

Project: Hackettstown

GII Project ID: 9225-11-19

		Date	Project No.	NMTL3107
Operator	Tch	16-Dec-19	Trial Pit No.	CBR23
Checked	Nc		Sample No.	B
Approved	Bc		Depth	0.50m

DETERMINATION OF THE CALIFORNIA BEARING RATIO TEST
BS 1377 : PART 4 : CLAUSE 7 : 1990

Soil Description Brown slightly gravelly silty SAND

Date 16-Dec-19

Test Method BS 1377: Part 4 : 1990 :7.4

Force Measuring Device VJT 08211

Test 1

Preparatic Remoulded with 2.5 kg rammer at natural moisture content

Surcharge 10 kPa

Mean Calibration 4.33

N/Div

Penetration Force Gauge

Force on 4.33

N/Div

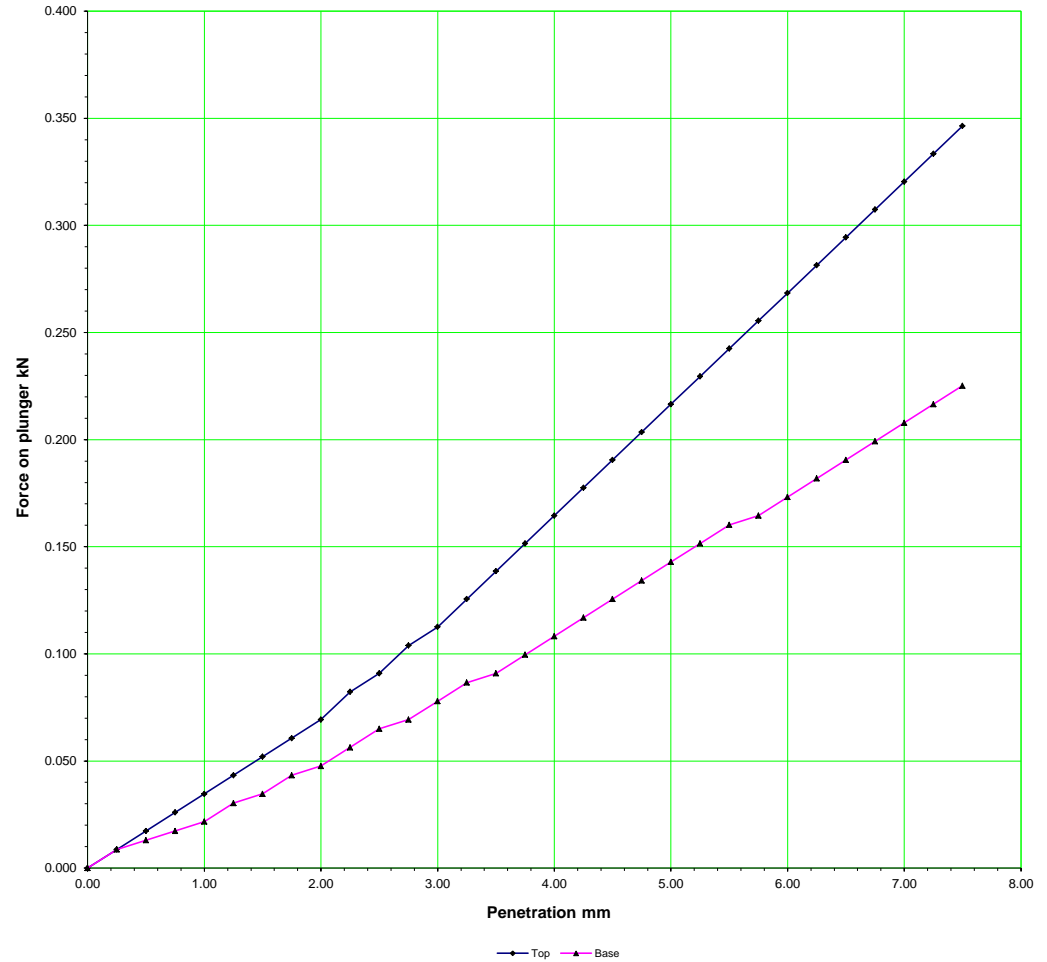
of plunger reading divisions

plunger kN

California Bearing Ratio Results %

mm	reading divisions		plunger kN		California Bearing Ratio Results %	
	Top	Bottom	Top	Bottom	Top	Base
0.00	0.0	0.0	0.000	0.000		
0.25	2.0	2.0	0.009	0.009		
0.50	4.0	3.0	0.017	0.013		
0.75	6.0	4.0	0.026	0.017		
1.00	8.0	5.0	0.035	0.022		
1.25	10.0	7.0	0.043	0.030		
1.50	12.0	8.0	0.052	0.035		
1.75	14.0	10.0	0.061	0.043		
2.00	16.0	11.0	0.069	0.048		
2.25	19.0	13.0	0.082	0.056		
2.50	21.0	15.0	0.091	0.065	0.69	0.49
2.75	24.0	16.0	0.104	0.069		
3.00	26.0	18.0	0.113	0.078		
3.25	29.0	20.0	0.126	0.087		
3.50	32.0	21.0	0.139	0.091		
3.75	35.0	23.0	0.152	0.100		
4.00	38.0	25.0	0.165	0.108		
4.25	41.0	27.0	0.178	0.117		
4.50	44.0	29.0	0.191	0.126		
4.75	47.0	31.0	0.204	0.134		
5.00	50.0	33.0	0.217	0.143	1.08	0.71
5.25	53.0	35.0	0.229	0.152		
5.50	56.0	37.0	0.242	0.160		
5.75	59.0	38.0	0.255	0.165		
6.00	62.0	40.0	0.268	0.173		
6.25	65.0	42.0	0.281	0.182		
6.50	68.0	44.0	0.294	0.191		
6.75	71.0	46.0	0.307	0.199		
7.00	74.0	48.0	0.320	0.208		
7.25	77.0	50.0	0.333	0.217		
7.50	80.0	52.0	0.346	0.225		

Moisture content after test		Top	Middle	Base	Specimen wt g	4745
Container No.		Tray	Tray	Tray	Diameter mm	152
Mass of wet soil + container	g	1810.0	1733.0	1670.5	Length mm	127.0
Mass of dry soil + container	g	1580.5	1522.4	1464.2		
Weight of container	g	144.0	187.6	144.0		
Mass of moisture	g	229.6	210.7	206.3	Average MC %	15.79
Dry weight	g	1436.5	1334.8	1320.2	Density Mg/m3	2.06
Moisture content	%	16.0	15.8	15.6	Dry Density Mg/m3	1.78



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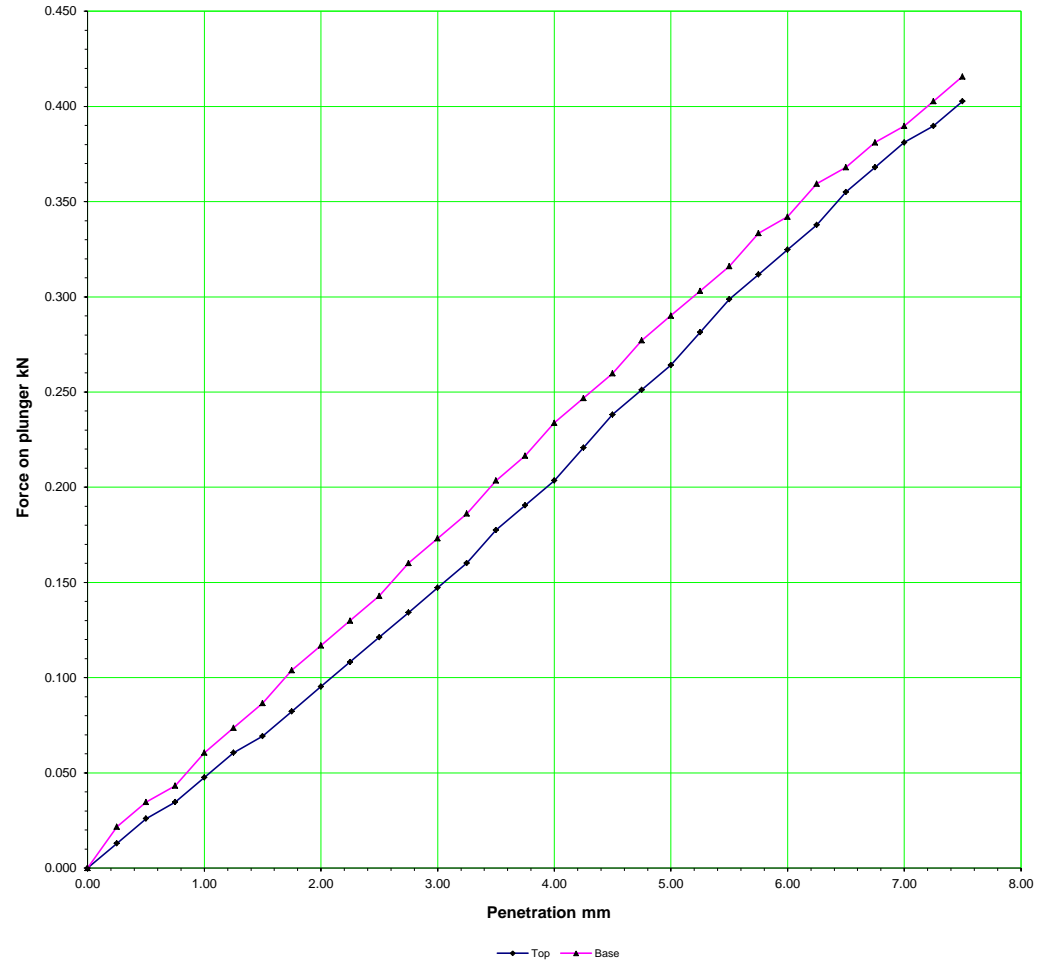
Project: Hackettstown

GII Project ID: 9225-11-19

		Date	Project No.	NMTL3107
Operator	Tch	16-Dec-19	Trial Pit No.	CBR24
Checked	Nc		Sample No.	B
Approved	Bc		Depth	0.50m

DETERMINATION OF THE CALIFORNIA BEARING RATIO TEST
BS 1377 : PART 4 : CLAUSE 7 : 1990

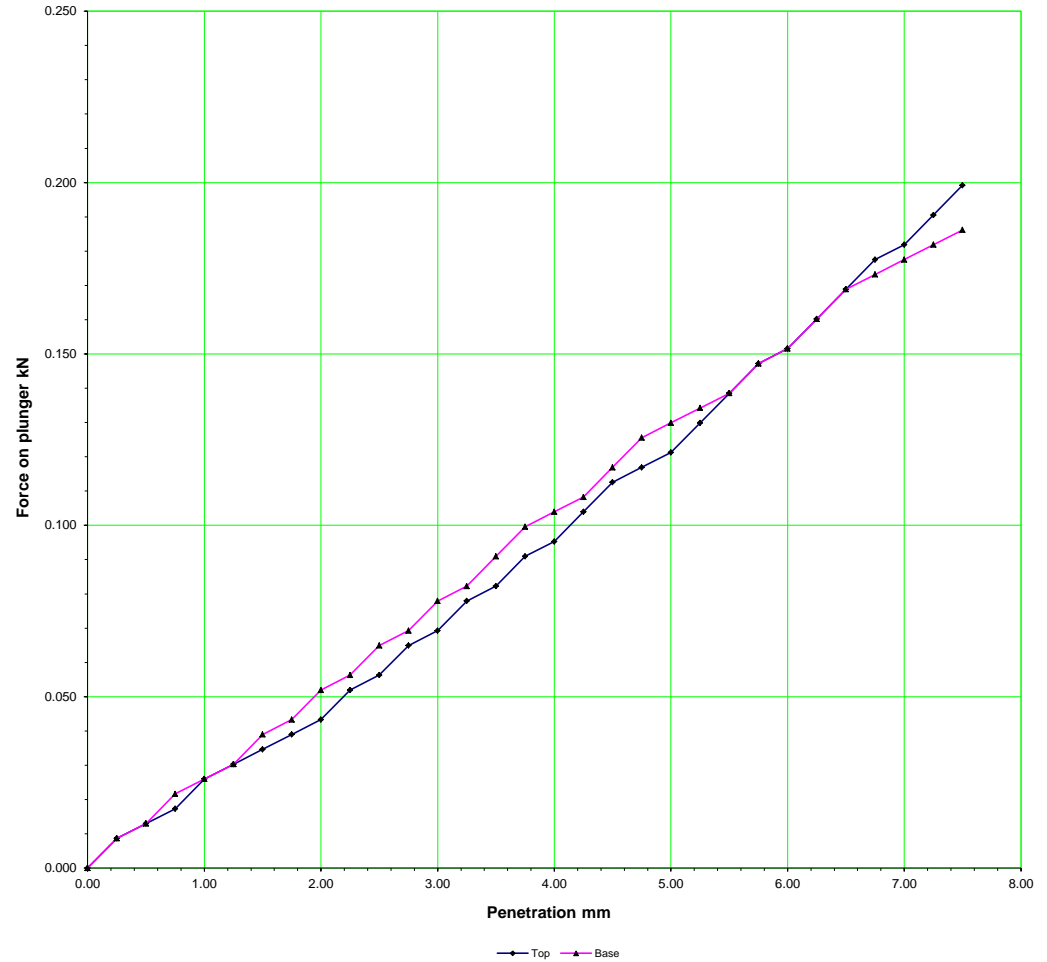
Soil Description	Brown slightly sandy slightly gravelly SILT/CLAY					Date	16-Dec-19	
Test Method	BS 1377: Part 4 : 1990 :7.4						Test 1	
Force Measuring Device	VJT 08211							
Preparatic Remoulded with 2.5 kg rammer at natural moisture content								
Surcharge	10 kPa		Mean Calibration		4.33	N/Div		
Penetration of plunger mm	Force Gauge reading divisions		Force on plunger kN		4.33	N/Div		
	Top	Bottom	Top	Bottom	Top	Base		
0.00	0.0	0.0	0.000	0.000				
0.25	3.0	5.0	0.013	0.022				
0.50	6.0	8.0	0.026	0.035				
0.75	8.0	10.0	0.035	0.043				
1.00	11.0	14.0	0.048	0.061				
1.25	14.0	17.0	0.061	0.074				
1.50	16.0	20.0	0.069	0.087				
1.75	19.0	24.0	0.082	0.104				
2.00	22.0	27.0	0.095	0.117				
2.25	25.0	30.0	0.108	0.130				
2.50	28.0	33.0	0.121	0.143	0.92	1.08		
2.75	31.0	37.0	0.134	0.160				
3.00	34.0	40.0	0.147	0.173				
3.25	37.0	43.0	0.160	0.186				
3.50	41.0	47.0	0.178	0.204				
3.75	44.0	50.0	0.191	0.217				
4.00	47.0	54.0	0.204	0.234				
4.25	51.0	57.0	0.221	0.247				
4.50	55.0	60.0	0.238	0.260				
4.75	58.0	64.0	0.251	0.277				
5.00	61.0	67.0	0.264	0.290	1.32	1.45		
5.25	65.0	70.0	0.281	0.303				
5.50	69.0	73.0	0.299	0.316				
5.75	72.0	77.0	0.312	0.333				
6.00	75.0	79.0	0.325	0.342				
6.25	78.0	83.0	0.338	0.359				
6.50	82.0	85.0	0.355	0.368				
6.75	85.0	88.0	0.368	0.381				
7.00	88.0	90.0	0.381	0.390				
7.25	90.0	93.0	0.390	0.403				
7.50	93.0	96.0	0.403	0.416				
Moisture content after test		Top	Middle	Base	Specimen wt g	4880		
Container No.		Tray	Tray	Tray	Diameter mm	152		
Mass of wet soil + container	g	1879.3	1871.9	1608.8	Length mm	127.0		
Mass of dry soil + container	g	1642.9	1642.3	1409.1				
Weight of container	g	148.1	188.7	145.4				
Mass of moisture	g	236.3	229.6	199.8	Average MC %	15.80		
Dry weight	g	1494.8	1453.6	1263.7	Density Mg/m3	2.12		
Moisture content	%	15.8	15.8	15.8	Dry Density Mg/m3	1.83		



<p>NM</p> <p>TL</p> <p>Ltd</p>	<p>Project: Hackettstown</p>	<p>Gil Project ID: 9225-11-19</p>	Date	16-Dec-19	Project No.	NMTL3107
			Operator	Tch	Trial Pit No.	CBR25
			Checked	Nc	Sample No.	B
			Approved	Bc	Depth	0.50m

DETERMINATION OF THE CALIFORNIA BEARING RATIO TEST
BS 1377 : PART 4 : CLAUSE 7 : 1990

Soil Description	Brown slightly sandy slightly gravelly SILT/CLAY					Date	16-Dec-19	
Test Method	BS 1377: Part 4 : 1990 :7.4						Test 1	
Force Measuring Device	VJT 08211							
Preparatic Remoulded with 2.5 kg rammer at natural moisture content								
Surcharge	10 kPa		Mean Calibration		4.33		N/Div	
Penetration of plunger	Force Gauge reading divisions		Force on plunger kN		4.33		N/Div	
mm	Top	Bottom	Top	Bottom	Top	California Bearing Ratio Results %		
						Base		
0.00	0.0	0.0	0.000	0.000				
0.25	2.0	2.0	0.009	0.009				
0.50	3.0	3.0	0.013	0.013				
0.75	4.0	5.0	0.017	0.022				
1.00	6.0	6.0	0.026	0.026				
1.25	7.0	7.0	0.030	0.030				
1.50	8.0	9.0	0.035	0.039				
1.75	9.0	10.0	0.039	0.043				
2.00	10.0	12.0	0.043	0.052				
2.25	12.0	13.0	0.052	0.056				
2.50	13.0	15.0	0.056	0.065	0.43	0.49		
2.75	15.0	16.0	0.065	0.069				
3.00	16.0	18.0	0.069	0.078				
3.25	18.0	19.0	0.078	0.082				
3.50	19.0	21.0	0.082	0.091				
3.75	21.0	23.0	0.091	0.100				
4.00	22.0	24.0	0.095	0.104				
4.25	24.0	25.0	0.104	0.108				
4.50	26.0	27.0	0.113	0.117				
4.75	27.0	29.0	0.117	0.126				
5.00	28.0	30.0	0.121	0.130	0.61	0.65		
5.25	30.0	31.0	0.130	0.134				
5.50	32.0	32.0	0.139	0.139				
5.75	34.0	34.0	0.147	0.147				
6.00	35.0	35.0	0.152	0.152				
6.25	37.0	37.0	0.160	0.160				
6.50	39.0	39.0	0.169	0.169				
6.75	41.0	40.0	0.178	0.173				
7.00	42.0	41.0	0.182	0.178				
7.25	44.0	42.0	0.191	0.182				
7.50	46.0	43.0	0.199	0.186				
Moisture content after test		Top	Middle	Base	Specimen wt g	4765		
Container No.		Tray	Tray	Tray	Diameter mm	152		
Mass of wet soil + container	g	1752.5	1950.0	1546.7	Length mm	127.0		
Mass of dry soil + container	g	1531.0	1698.0	1346.0				
Weight of container	g	192.4	187.3	146.0				
Mass of moisture	g	221.5	252.1	200.7	Average MC %	16.65		
Dry weight	g	1338.6	1510.6	1200.0	Density Mg/m3	2.07		
Moisture content	%	16.5	16.7	16.7	Dry Density Mg/m3	1.77		



NM TL Ltd	Project: Hackettstown	Gil Project ID: 9225-11-19	Date		Project No.	NMTL3107	
			Operator	Tch	16-Dec-19	Trial Pit No.	CBR26
			Checked	Nc		Sample No.	B
			Approved	Bc		Depth	0.50m

POINT LOAD STRENGTH INDEX TESTS

ISRM : 1989

BH No.	Depth m	Type/Orientation	Width (W) mm	Platen Separation (D) mm Measured	Failure Load (P) kN	De2 mm2	Point Load Index(Is) Mn/m2	IS(50) Mpa	UCS Mpa
BH02	10.15-10.35	D	195.0	63.34	15.0	4012	3.738825	4.208	84.16
BH02	10.15-10.35	A	63.3	97.93	17.0	12211	1.392218	4.773	95.45
BH04	11.4-11.60	D	97.9	63.31	5.0	4008	1.247456	1.404	28.07
BH04	11.4-11.60	A	63.3	49.66	5.0	3140	1.592379	1.404	28.07
RC08	12.2-12.4	D	203.0	63.40	18.5	4020	4.602494	5.183	103.65
RC08	12.2-12.4	A	63.4	77.80	19.4	7707	2.517285	5.435	108.70
RC08	12.6-12.7	D	96.8	63.92	11.0	4086	2.692273	3.044	60.88
RC08	12.6-12.7	A	63.9	88.65	16.0	10006	1.599014	4.428	88.55
RC09	13.1-13.3	D	198.0	63.60	16.0	4045	3.955540	4.461	89.22
RC09	13.1-13.3	A	63.6	81.82	19.2	8524	2.252538	5.353	107.07
RC09	15.0-15.2	D	201.0	63.66	19.0	4053	4.688353	5.290	105.80
RC09	15.0-15.2	A	63.7	98.25	17.0	12291	1.383164	4.733	94.67

- Notes:
1. Specimens tested as received from site.
 2. Orientation of specimens is not known.
 3. D= Diameterial
 4. A= Axial

NMTL Ltd	Project Name:	Hackettstown	Project No.	NMTL 3196	Tested	09/06/2020	Signed
				Tch		Bc	

Ground Investigations Ireland
Catherinestown House
Hazelhatch Road
Newcastle
Co. Dublin
Ireland



Attention : Mike Sutton
Date : 16th December, 2019
Your reference : 9225-11-19
Our reference : Test Report 19/19841 Batch 1
Location : Hackettstown
Date samples received : 4th December, 2019
Status : Final report
Issue : 1

Three samples were received for analysis on 4th December, 2019 of which three were scheduled for analysis. Please find attached our Test Report which should be read with notes at the end of the report and should include all sections if reproduced. Interpretations and opinions are outside the scope of any accreditation, and all results relate only to samples supplied.

All analysis is carried out on as received samples and reported on a dry weight basis unless stated otherwise. Results are not surrogate corrected.

Authorised By:



Phil Sommerton BSc
Senior Project Manager

Please include all sections of this report if it is reproduced

Client Name: Ground Investigations Ireland
Reference: 19/11/9225
Location: Hackettstown
Contact: Mike Sutton

Note:
 Asbestos Screen analysis is carried out in accordance with our documented in-house methods PM042 and TM065 and HSG 248 by Stereo and Polarised Light Microscopy using Dispersion Staining Techniques and is covered by our UKAS accreditation. Detailed Gravimetric Quantification and PCOM Fibre Analysis is carried out in accordance with our documented in-house methods PM042 and TM131 and HSG 248 using Stereo and Polarised Light Microscopy and Phase Contrast Optical Microscopy (PCOM). Samples are retained for not less than 6 months from the date of analysis unless specifically requested.

Opinions, including ACM type and Asbestos level less than 0.1%, lie outside the scope of our UKAS accreditation.

Where the sample is not taken by a Element Materials Technology consultant, Element Materials Technology cannot be responsible for inaccurate or unrepresentative sampling.

EMT Job No.	Batch	Sample ID	Depth	EMT Sample No.	Date Of Analysis	Analysis	Result
19/19841	1	TP05	0.50	2	05/12/2019	General Description (Bulk Analysis)	Soil/Stone
					05/12/2019	Asbestos Fibres	NAD
					05/12/2019	Asbestos ACM	NAD
					05/12/2019	Asbestos Type	NAD
					05/12/2019	Asbestos Level Screen	NAD
19/19841	1	TP06	0.50	5	05/12/2019	General Description (Bulk Analysis)	Soil/Stone
					05/12/2019	Asbestos Fibres	NAD
					05/12/2019	Asbestos ACM	NAD
					05/12/2019	Asbestos Type	NAD
					05/12/2019	Asbestos Level Screen	NAD
19/19841	1	TP10	0.50	8	05/12/2019	General Description (Bulk Analysis)	Soil/Stone
					05/12/2019	Asbestos Fibres	NAD
					05/12/2019	Asbestos ACM	NAD
					05/12/2019	Asbestos Type	NAD
					05/12/2019	Asbestos Level Screen	NAD

NOTES TO ACCOMPANY ALL SCHEDULES AND REPORTS

EMT Job No.: 19/19841

SOILS

Please note we are only MCERTS accredited (UK soils only) for sand, loam and clay and any other matrix is outside our scope of accreditation.

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation has been performed on clay, sand and loam, only samples that are predominantly these matrices, or combinations of them will be within our MCERTS scope. If samples are not one of a combination of the above matrices they will not be marked as MCERTS accredited.

It is assumed that you have taken representative samples on site and require analysis on a representative subsample. Stones will generally be included unless we are requested to remove them.

All samples will be discarded one month after the date of reporting, unless we are instructed to the contrary.

If you have not already done so, please send us a purchase order if this is required by your company.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

All analysis is reported on a dry weight basis unless stated otherwise. Limits of detection for analyses carried out on as received samples are not moisture content corrected. Results are not surrogate corrected. Samples are dried at 35°C ±5°C unless otherwise stated. Moisture content for CEN Leachate tests are dried at 105°C ±5°C.

Where Mineral Oil or Fats, Oils and Grease is quoted, this refers to Total Aliphatics C10-C40.

Where a CEN 10:1 ZERO Headspace VOC test has been carried out, a 10:1 ratio of water to wet (as received) soil has been used.

% Asbestos in Asbestos Containing Materials (ACMs) is determined by reference to HSG 264 The Survey Guide - Appendix 2 : ACMs in buildings listed in order of ease of fibre release.

Sufficient amount of sample must be received to carry out the testing specified. Where an insufficient amount of sample has been received the testing may not meet the requirements of our accredited methods, as such accreditation may be removed.

Negative Neutralization Potential (NP) values are obtained when the volume of NaOH (0.1N) titrated (pH 8.3) is greater than the volume of HCl (1N) to reduce the pH of the sample to 2.0 - 2.5. Any negative NP values are corrected to 0.

The calculation of Pyrite content assumes that all oxidisable sulphides present in the sample are pyrite. This may not be the case. The calculation may be an overestimate when other sulphides such as Barite (Barium Sulphate) are present.

WATERS

Please note we are not a UK Drinking Water Inspectorate (DWI) Approved Laboratory .

ISO17025 accreditation applies to surface water and groundwater and usually one other matrix which is analysis specific, any other liquids are outside our scope of accreditation.

As surface waters require different sample preparation to groundwaters the laboratory must be informed of the water type when submitting samples.

Where Mineral Oil or Fats, Oils and Grease is quoted, this refers to Total Aliphatics C10-C40.

DEVIATING SAMPLES

All samples should be submitted to the laboratory in suitable containers with sufficient ice packs to sustain an appropriate temperature for the requested analysis. The temperature of sample receipt is recorded on the confirmation schedules in order that the client can make an informed decision as to whether testing should still be undertaken.

SURROGATES

Surrogate compounds are added during the preparation process to monitor recovery of analytes. However low recovery in soils is often due to peat, clay or other organic rich matrices. For waters this can be due to oxidants, surfactants, organic rich sediments or remediation fluids. Acceptable limits for most organic methods are 70 - 130% and for VOCs are 50 - 150%. When surrogate recoveries are outside the performance criteria but the associated AQC passes this is assumed to be due to matrix effect. Results are not surrogate corrected.

DILUTIONS

A dilution suffix indicates a dilution has been performed and the reported result takes this into account. No further calculation is required.

BLANKS

Where analytes have been found in the blank, the sample will be treated in accordance with our laboratory procedure for dealing with contaminated blanks.

NOTE

Data is only reported if the laboratory is confident that the data is a true reflection of the samples analysed. Data is only reported as accredited when all the requirements of our Quality System have been met. In certain circumstances where all the requirements of the Quality System have not been met, for instance if the associated AQC has failed, the reason is fully investigated and documented. The sample data is then evaluated alongside the other quality control checks performed during analysis to determine its suitability. Following this evaluation, provided the sample results have not been effected, the data is reported but accreditation is removed. It is a UKAS requirement for data not reported as accredited to be considered indicative only, but this does not mean the data is not valid.

Where possible, and if requested, samples will be re-extracted and a revised report issued with accredited results. Please do not hesitate to contact the laboratory if further details are required of the circumstances which have led to the removal of accreditation.

Please include all sections of this report if it is reproduced

REPORTS FROM THE SOUTH AFRICA LABORATORY

Any method number not prefixed with SA has been undertaken in our UK laboratory unless reported as subcontracted.

Measurement Uncertainty

Measurement uncertainty defines the range of values that could reasonably be attributed to the measured quantity. This range of values has not been included within the reported results. Uncertainty expressed as a percentage can be provided upon request.

ABBREVIATIONS and ACRONYMS USED

#	ISO17025 (UKAS Ref No. 4225) accredited - UK.
SA	ISO17025 (SANAS Ref No.T0729) accredited - South Africa
B	Indicates analyte found in associated method blank.
DR	Dilution required.
M	MCERTS accredited.
NA	Not applicable
NAD	No Asbestos Detected.
ND	None Detected (usually refers to VOC and/SVOC TICs).
NDP	No Determination Possible
SS	Calibrated against a single substance
SV	Surrogate recovery outside performance criteria. This may be due to a matrix effect.
W	Results expressed on as received basis.
+	AQC failure, accreditation has been removed from this result, if appropriate, see 'Note' on previous page.
>>	Results above calibration range, the result should be considered the minimum value. The actual result could be significantly higher, this result is not accredited.
*	Analysis subcontracted to an Element Materials Technology approved laboratory.
AD	Samples are dried at 35°C ±5°C
CO	Suspected carry over
LOD/LOR	Limit of Detection (Limit of Reporting) in line with ISO 17025 and MCERTS
ME	Matrix Effect
NFD	No Fibres Detected
BS	AQC Sample
LB	Blank Sample
N	Client Sample
TB	Trip Blank Sample
OC	Outside Calibration Range

EMT Job No: 19/19841

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/S ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
PM4	Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465 and BS1377.	PM0	No preparation is required.			AR	
TM4	Modified USEPA 8270 method for the solvent extraction and determination of PAHs by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.			AR	Yes
TM4	Modified USEPA 8270 method for the solvent extraction and determination of PAHs by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.	Yes		AR	Yes
TM5	Modified 8015B method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM16	Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE.			AR	
TM5	Modified 8015B method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM8/PM16	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE.			AR	Yes
TM5	Modified 8015B method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM8/PM16	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required/Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE.	Yes		AR	Yes
TM5/TM36	please refer to TM5 and TM36 for method details	PM8/PM12/PM16	please refer to PM8/PM16 and PM12 for method details			AR	Yes
TM17	Modified US EPA method 8270. Determination of specific Polychlorinated Biphenyl congeners by GC-MS.	PM8	End over end extraction of solid samples for organic analysis. The solvent mix varies depending on analysis required.	Yes		AR	Yes
TM20	Modified BS 1377-3: 1990/USEPA 160.3 Gravimetric determination of Total Dissolved Solids/Total Solids	PM0	No preparation is required.	Yes		AR	Yes
TM21	Modified BS 7755-3:1995, ISO10694:1995 Determination of Total Organic Carbon or Total Carbon by combustion in an Eltra TOC furnace/analyser in the presence of oxygen. The CO2 generated is quantified using infra-red detection. Organic Matter (SOM) calculated as per EA MCERTS Chemical Testing of Soil, March 2012 v4.	PM24	Dried and ground solid samples are washed with hydrochloric acid, then rinsed with deionised water to remove the mineral carbon before TOC analysis.	Yes		AD	Yes

EMT Job No: 19/19841

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/S ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
TM26	Determination of phenols by Reversed Phased High Performance Liquid Chromatography and Electro-Chemical Detection.	PM0	No preparation is required.			AR	Yes
TM30	Determination of Trace Metal elements by ICP-OES (Inductively Coupled Plasma - Optical Emission Spectrometry). Modified US EPA Method 200.7, 6010B and BS EN ISO 11885 2009	PM15	Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground.			AD	Yes
TM30	Determination of Trace Metal elements by ICP-OES (Inductively Coupled Plasma - Optical Emission Spectrometry). Modified US EPA Method 200.7, 6010B and BS EN ISO 11885 2009	PM15	Acid digestion of dried and ground solid samples using Aqua Regia refluxed at 112.5 °C. Samples containing asbestos are not dried and ground.	Yes		AD	Yes
TM30	Determination of Trace Metal elements by ICP-OES (Inductively Coupled Plasma - Optical Emission Spectrometry). Modified US EPA Method 200.7, 6010B and BS EN ISO 11885 2009	PM17	Modified method BS EN12457-2 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio.	Yes		AR	Yes
TM31	Modified USEPA 8015B. Determination of Methylterbutylether, Benzene, Toluene, Ethylbenzene and Xylene by headspace GC-FID.	PM12	Modified US EPA method 5021. Preparation of solid and liquid samples for GC headspace analysis.			AR	Yes
TM31	Modified USEPA 8015B. Determination of Methylterbutylether, Benzene, Toluene, Ethylbenzene and Xylene by headspace GC-FID.	PM12	Modified US EPA method 5021. Preparation of solid and liquid samples for GC headspace analysis.	Yes		AR	Yes
TM36	Modified US EPA method 8015B. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GCFID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results can be confirmed using GCMS.	PM12	Modified US EPA method 5021. Preparation of solid and liquid samples for GC headspace analysis.			AR	Yes
TM36	Modified US EPA method 8015B. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GCFID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE results can be confirmed using GCMS.	PM12	Modified US EPA method 5021. Preparation of solid and liquid samples for GC headspace analysis.	Yes		AR	Yes
TM38	Soluble Ion analysis using Discrete Analyser. Modified US EPA methods 325.2 (Chloride), 375.4 (Sulphate), 365.2 (o-Phosphate), 353.1 (TON), 354.1 (Nitrite), 350.1 (NH4+) comparable to BS ISO 15923-1, 7196A (Hex Cr)	PM0	No preparation is required.	Yes		AR	Yes
TM38	Soluble Ion analysis using Discrete Analyser. Modified US EPA methods 325.2 (Chloride), 375.4 (Sulphate), 365.2 (o-Phosphate), 353.1 (TON), 354.1 (Nitrite), 350.1 (NH4+) comparable to BS ISO 15923-1, 7196A (Hex Cr)	PM20	Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker.	Yes		AR	Yes

EMT Job No: 19/19841

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/S ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
TM60	TC/TOC analysis of Waters by High Temperature Combustion followed by NDIR detection. Based on the following modified standard methods: USEPA 9060, APHA Standard Methods for Examination of Water and Wastewater 5310B, ASTM D 7573, and USEPA 415.1.	PM0	No preparation is required.			AR	Yes
TM61	Modified US EPA methods 245.7 and 200.7. Determination of Mercury by Cold Vapour Atomic Fluorescence.	PM0	No preparation is required.	Yes		AR	Yes
TM65	Asbestos Bulk Identification method based on HSG 248.	PM42	Solid samples undergo a thorough visual inspection for asbestos fibres prior to asbestos identification using TM065.	Yes		AR	
TM73	Modified US EPA methods 150.1 and 9045D and BS1377:1990. Determination of pH by Metrohm automated probe analyser.	PM0	No preparation is required.			AR	Yes
TM73	Modified US EPA methods 150.1 and 9045D and BS1377:1990. Determination of pH by Metrohm automated probe analyser.	PM11	Extraction of as received solid samples using one part solid to 2.5 parts deionised water.	Yes		AR	No
TM173	Analysis of fluoride by ISE (Ion Selective Electrode) using modified ISE method 340.2	PM0	No preparation is required.			AR	Yes
NONE	No Method Code	NONE	No Method Code			AD	Yes
NONE	No Method Code	PM17	Modified method BS EN12457-2 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio.				
NONE	No Method Code	PM17	Modified method BS EN12457-2 As received solid samples are leached with water in a 10:1 water to soil ratio for 24 hours, the moisture content of the sample is included in the ratio.			AR	
NONE	No Method Code	PM4	Gravimetric measurement of Natural Moisture Content and % Moisture Content at either 35°C or 105°C. Calculation based on ISO 11465 and BS1377.			AR	

Ground Investigations Ireland
Catherinestown House
Hazelhatch Road
Newcastle
Co. Dublin
Ireland



Attention : Mike Sutton
Date : 13th December, 2019
Your reference : 9225-11-19
Our reference : Test Report 19/19816 Batch 1
Location : Hackettstown
Date samples received : 4th December, 2019
Status : Final report
Issue : 1

Two samples were received for analysis on 4th December, 2019 of which two were scheduled for analysis. Please find attached our Test Report which should be read with notes at the end of the report and should include all sections if reproduced. Interpretations and opinions are outside the scope of any accreditation, and all results relate only to samples supplied.
All analysis is carried out on as received samples and reported on a dry weight basis unless stated otherwise. Results are not surrogate corrected.

Authorised By:



Phil Sommerton BSc
Senior Project Manager

Please include all sections of this report if it is reproduced

NOTES TO ACCOMPANY ALL SCHEDULES AND REPORTS

EMT Job No.: 19/19816

SOILS

Please note we are only MCERTS accredited (UK soils only) for sand, loam and clay and any other matrix is outside our scope of accreditation.

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation has been performed on clay, sand and loam, only samples that are predominantly these matrices, or combinations of them will be within our MCERTS scope. If samples are not one of a combination of the above matrices they will not be marked as MCERTS accredited.

It is assumed that you have taken representative samples on site and require analysis on a representative subsample. Stones will generally be included unless we are requested to remove them.

All samples will be discarded one month after the date of reporting, unless we are instructed to the contrary.

If you have not already done so, please send us a purchase order if this is required by your company.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

All analysis is reported on a dry weight basis unless stated otherwise. Limits of detection for analyses carried out on as received samples are not moisture content corrected. Results are not surrogate corrected. Samples are dried at 35°C ±5°C unless otherwise stated. Moisture content for CEN Leachate tests are dried at 105°C ±5°C.

Where Mineral Oil or Fats, Oils and Grease is quoted, this refers to Total Aliphatics C10-C40.

Where a CEN 10:1 ZERO Headspace VOC test has been carried out, a 10:1 ratio of water to wet (as received) soil has been used.

% Asbestos in Asbestos Containing Materials (ACMs) is determined by reference to HSG 264 The Survey Guide - Appendix 2 : ACMs in buildings listed in order of ease of fibre release.

Sufficient amount of sample must be received to carry out the testing specified. Where an insufficient amount of sample has been received the testing may not meet the requirements of our accredited methods, as such accreditation may be removed.

Negative Neutralization Potential (NP) values are obtained when the volume of NaOH (0.1N) titrated (pH 8.3) is greater than the volume of HCl (1N) to reduce the pH of the sample to 2.0 - 2.5. Any negative NP values are corrected to 0.

The calculation of Pyrite content assumes that all oxidisable sulphides present in the sample are pyrite. This may not be the case. The calculation may be an overestimate when other sulphides such as Barite (Barium Sulphate) are present.

WATERS

Please note we are not a UK Drinking Water Inspectorate (DWI) Approved Laboratory .

ISO17025 accreditation applies to surface water and groundwater and usually one other matrix which is analysis specific, any other liquids are outside our scope of accreditation.

As surface waters require different sample preparation to groundwaters the laboratory must be informed of the water type when submitting samples.

Where Mineral Oil or Fats, Oils and Grease is quoted, this refers to Total Aliphatics C10-C40.

DEVIATING SAMPLES

All samples should be submitted to the laboratory in suitable containers with sufficient ice packs to sustain an appropriate temperature for the requested analysis. The temperature of sample receipt is recorded on the confirmation schedules in order that the client can make an informed decision as to whether testing should still be undertaken.

SURROGATES

Surrogate compounds are added during the preparation process to monitor recovery of analytes. However low recovery in soils is often due to peat, clay or other organic rich matrices. For waters this can be due to oxidants, surfactants, organic rich sediments or remediation fluids. Acceptable limits for most organic methods are 70 - 130% and for VOCs are 50 - 150%. When surrogate recoveries are outside the performance criteria but the associated AQC passes this is assumed to be due to matrix effect. Results are not surrogate corrected.

DILUTIONS

A dilution suffix indicates a dilution has been performed and the reported result takes this into account. No further calculation is required.

BLANKS

Where analytes have been found in the blank, the sample will be treated in accordance with our laboratory procedure for dealing with contaminated blanks.

NOTE

Data is only reported if the laboratory is confident that the data is a true reflection of the samples analysed. Data is only reported as accredited when all the requirements of our Quality System have been met. In certain circumstances where all the requirements of the Quality System have not been met, for instance if the associated AQC has failed, the reason is fully investigated and documented. The sample data is then evaluated alongside the other quality control checks performed during analysis to determine its suitability. Following this evaluation, provided the sample results have not been effected, the data is reported but accreditation is removed. It is a UKAS requirement for data not reported as accredited to be considered indicative only, but this does not mean the data is not valid.

Where possible, and if requested, samples will be re-extracted and a revised report issued with accredited results. Please do not hesitate to contact the laboratory if further details are required of the circumstances which have led to the removal of accreditation.

Please include all sections of this report if it is reproduced

REPORTS FROM THE SOUTH AFRICA LABORATORY

Any method number not prefixed with SA has been undertaken in our UK laboratory unless reported as subcontracted.

Measurement Uncertainty

Measurement uncertainty defines the range of values that could reasonably be attributed to the measured quantity. This range of values has not been included within the reported results. Uncertainty expressed as a percentage can be provided upon request.

ABBREVIATIONS and ACRONYMS USED

#	ISO17025 (UKAS Ref No. 4225) accredited - UK.
SA	ISO17025 (SANAS Ref No.T0729) accredited - South Africa
B	Indicates analyte found in associated method blank.
DR	Dilution required.
M	MCERTS accredited.
NA	Not applicable
NAD	No Asbestos Detected.
ND	None Detected (usually refers to VOC and/SVOC TICs).
NDP	No Determination Possible
SS	Calibrated against a single substance
SV	Surrogate recovery outside performance criteria. This may be due to a matrix effect.
W	Results expressed on as received basis.
+	AQC failure, accreditation has been removed from this result, if appropriate, see 'Note' on previous page.
>>	Results above calibration range, the result should be considered the minimum value. The actual result could be significantly higher, this result is not accredited.
*	Analysis subcontracted to an Element Materials Technology approved laboratory.
AD	Samples are dried at 35°C ±5°C
CO	Suspected carry over
LOD/LOR	Limit of Detection (Limit of Reporting) in line with ISO 17025 and MCERTS
ME	Matrix Effect
NFD	No Fibres Detected
BS	AQC Sample
LB	Blank Sample
N	Client Sample
TB	Trip Blank Sample
OC	Outside Calibration Range

EMT Job No: 19/19816

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/S ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
TM38	Soluble Ion analysis using Discrete Analyser. Modified US EPA methods 325.2 (Chloride), 375.4 (Sulphate), 365.2 (o-Phosphate), 353.1 (TON), 354.1 (Nitrite), 350.1 (NH4+) comparable to BS ISO 15923-1, 7196A (Hex Cr)	PM20	Extraction of dried and ground or as received samples with deionised water in a 2:1 water to solid ratio using a reciprocal shaker for all analytes except hexavalent chromium. Extraction of as received sample using 10:1 ratio of 0.2M sodium hydroxide to soil for hexavalent chromium using a reciprocal shaker.	Yes		AD	Yes
TM73	Modified US EPA methods 150.1 and 9045D and BS1377:1990. Determination of pH by Metrohm automated probe analyser.	PM11	Extraction of as received solid samples using one part solid to 2.5 parts deionised water.	Yes		AR	No

Ground Investigations Ireland
Catherinestown House
Hazelhatch Road
Newcastle
Co. Dublin
Ireland



Attention : Mike Sutton
Date : 1st June, 2020
Your reference : 9225-11-19
Our reference : Test Report 20/6499 Batch 1
Location : Hackettstown, Skerries
Date samples received : 22nd May, 2020
Status : Final report
Issue : 1

Four samples were received for analysis on 22nd May, 2020 of which four were scheduled for analysis. Please find attached our Test Report which should be read with notes at the end of the report and should include all sections if reproduced. Interpretations and opinions are outside the scope of any accreditation, and all results relate only to samples supplied.
All analysis is carried out on as received samples and reported on a dry weight basis unless stated otherwise. Results are not surrogate corrected.

Authorised By:



Phil Sommerton BSc

Senior Project Manager

Please include all sections of this report if it is reproduced

NOTES TO ACCOMPANY ALL SCHEDULES AND REPORTS

EMT Job No.: 20/6499

SOILS

Please note we are only MCERTS accredited (UK soils only) for sand, loam and clay and any other matrix is outside our scope of accreditation.

Where an MCERTS report has been requested, you will be notified within 48 hours of any samples that have been identified as being outside our MCERTS scope. As validation has been performed on clay, sand and loam, only samples that are predominantly these matrices, or combinations of them will be within our MCERTS scope. If samples are not one of a combination of the above matrices they will not be marked as MCERTS accredited.

It is assumed that you have taken representative samples on site and require analysis on a representative subsample. Stones will generally be included unless we are requested to remove them.

All samples will be discarded one month after the date of reporting, unless we are instructed to the contrary.

If you have not already done so, please send us a purchase order if this is required by your company.

Where appropriate please make sure that our detection limits are suitable for your needs, if they are not, please notify us immediately.

All analysis is reported on a dry weight basis unless stated otherwise. Limits of detection for analyses carried out on as received samples are not moisture content corrected. Results are not surrogate corrected. Samples are dried at 35°C ±5°C unless otherwise stated. Moisture content for CEN Leachate tests are dried at 105°C ±5°C.

Where Mineral Oil or Fats, Oils and Grease is quoted, this refers to Total Aliphatics C10-C40.

Where a CEN 10:1 ZERO Headspace VOC test has been carried out, a 10:1 ratio of water to wet (as received) soil has been used.

% Asbestos in Asbestos Containing Materials (ACMs) is determined by reference to HSG 264 The Survey Guide - Appendix 2 : ACMs in buildings listed in order of ease of fibre release.

Sufficient amount of sample must be received to carry out the testing specified. Where an insufficient amount of sample has been received the testing may not meet the requirements of our accredited methods, as such accreditation may be removed.

Negative Neutralization Potential (NP) values are obtained when the volume of NaOH (0.1N) titrated (pH 8.3) is greater than the volume of HCl (1N) to reduce the pH of the sample to 2.0 - 2.5. Any negative NP values are corrected to 0.

The calculation of Pyrite content assumes that all oxidisable sulphides present in the sample are pyrite. This may not be the case. The calculation may be an overestimate when other sulphides such as Barite (Barium Sulphate) are present.

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As surface waters require different sample preparation to groundwaters the laboratory must be informed of the water type when submitting samples.

Where Mineral Oil or Fats, Oils and Grease is quoted, this refers to Total Aliphatics C10-C40.

DEVIATING SAMPLES

All samples should be submitted to the laboratory in suitable containers with sufficient ice packs to sustain an appropriate temperature for the requested analysis. The temperature of sample receipt is recorded on the confirmation schedules in order that the client can make an informed decision as to whether testing should still be undertaken.

SURROGATES

Surrogate compounds are added during the preparation process to monitor recovery of analytes. However low recovery in soils is often due to peat, clay or other organic rich matrices. For waters this can be due to oxidants, surfactants, organic rich sediments or remediation fluids. Acceptable limits for most organic methods are 70 - 130% and for VOCs are 50 - 150%. When surrogate recoveries are outside the performance criteria but the associated AQC passes this is assumed to be due to matrix effect. Results are not surrogate corrected.

DILUTIONS

A dilution suffix indicates a dilution has been performed and the reported result takes this into account. No further calculation is required.

BLANKS

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Where possible, and if requested, samples will be re-extracted and a revised report issued with accredited results. Please do not hesitate to contact the laboratory if further details are required of the circumstances which have led to the removal of accreditation.

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REPORTS FROM THE SOUTH AFRICA LABORATORY

Any method number not prefixed with SA has been undertaken in our UK laboratory unless reported as subcontracted.

Measurement Uncertainty

Measurement uncertainty defines the range of values that could reasonably be attributed to the measured quantity. This range of values has not been included within the reported results. Uncertainty expressed as a percentage can be provided upon request.

ABBREVIATIONS and ACRONYMS USED

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SA	ISO17025 (SANAS Ref No.T0729) accredited - South Africa
B	Indicates analyte found in associated method blank.
DR	Dilution required.
M	MCERTS accredited.
NA	Not applicable
NAD	No Asbestos Detected.
ND	None Detected (usually refers to VOC and/SVOC TICs).
NDP	No Determination Possible
SS	Calibrated against a single substance
SV	Surrogate recovery outside performance criteria. This may be due to a matrix effect.
W	Results expressed on as received basis.
+	AQC failure, accreditation has been removed from this result, if appropriate, see 'Note' on previous page.
>>	Results above calibration range, the result should be considered the minimum value. The actual result could be significantly higher, this result is not accredited.
*	Analysis subcontracted to an Element Materials Technology approved laboratory.
AD	Samples are dried at 35°C ±5°C
CO	Suspected carry over
LOD/LOR	Limit of Detection (Limit of Reporting) in line with ISO 17025 and MCERTS
ME	Matrix Effect
NFD	No Fibres Detected
BS	AQC Sample
LB	Blank Sample
N	Client Sample
TB	Trip Blank Sample
OC	Outside Calibration Range

EMT Job No: 20/6499

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/S ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
TM4	Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS.	PM30	Water samples are extracted with solvent using a magnetic stirrer to create a vortex.				
TM4	Modified USEPA 8270D v5:2014 method for the solvent extraction and determination of PAHs by GC-MS.	PM30	Water samples are extracted with solvent using a magnetic stirrer to create a vortex.	Yes			
TM5	Modified 8015B v2:1996 method for the determination of solvent Extractable Petroleum Hydrocarbons (EPH) within the range C8-C40 by GCFID. For waters the solvent extracts dissolved phase plus a sheen if present.	PM16/PM30	Fractionation into aliphatic and aromatic fractions using a Rapid Trace SPE/Water samples are extracted with solvent using a magnetic stirrer to create a vortex.	Yes			
TM5/TM36	please refer to TM5 and TM36 for method details	PM12/PM16/PM30	please refer to PM16/PM30 and PM12 for method details	Yes			
TM26	Determination of phenols by Reversed Phased High Performance Liquid Chromatography and Electro-Chemical Detection.	PM0	No preparation is required.				
TM30	Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry); WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP	PM14	Preparation of waters and leachates for metals by ICP OES/ICP MS. Samples are filtered for Dissolved metals, and remain unfiltered for Total metals then acidified				
TM30	Determination of Trace Metals by ICP-OES (Inductively Coupled Plasma – Optical Emission Spectrometry); WATERS by Modified USEPA Method 200.7, Rev. 4.4, 1994; Modified EPA Method 6010B, Rev.2, Dec 1996; Modified BS EN ISO 11885:2009: SOILS by Modified USEP	PM14	Preparation of waters and leachates for metals by ICP OES/ICP MS. Samples are filtered for Dissolved metals, and remain unfiltered for Total metals then acidified	Yes			
TM36	Modified US EPA method 8015B v2:1996. Determination of Gasoline Range Organics (GRO) in the carbon chain range of C4-12 by headspace GC-FID. MTBE by GCFID co-elutes with 3-methylpentane if present and therefore can give a false positive. Positive MTBE re	PM12	Modified US EPA method 5021A v2:2014. Preparation of solid and liquid samples for GC headspace analysis.	Yes			
TM38	Soluble Ion analysis using Discrete Analyser. Modified US EPA methods: Chloride 325.2 (1978), Sulphate 375.4 (Rev.2 1993), o-Phosphate 365.2 (Rev.2 1993), TON 353.1 (Rev.2 1993), Nitrite 354.1 (1971), Hex Cr 7196A (1992), NH4+ 350.1 (Rev.2 1993 (comparabl	PM0	No preparation is required.	Yes			
TM73	Modified US EPA methods 150.1 (1982) and 9045D Rev. 4 - 2004) and BS1377-3:1990. Determination of pH by Metrohm automated probe analyser.	PM0	No preparation is required.	Yes			

EMT Job No: 20/6499

Test Method No.	Description	Prep Method No. (if appropriate)	Description	ISO 17025 (UKAS/S ANAS)	MCERTS (UK soils only)	Analysis done on As Received (AR) or Dried (AD)	Reported on dry weight basis
TM75	Modified US EPA method 310.1 (1978). Determination of Alkalinity by Metrohm automated titration analyser.	PM0	No preparation is required.				
TM76	Modified US EPA method 120.1 (1982). Determination of Specific Conductance by Metrohm automated probe analyser.	PM0	No preparation is required.	Yes			
TM89	Modified USEPA method OIA-1667 (1999). Determination of cyanide by Flow Injection Analyser. Where WAD cyanides are required a Ligand displacement step is carried out before analysis.	PM0	No preparation is required.	Yes			

APPENDIX 7 – Groundwater Monitoring





GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

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D22 YD52

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Web: www.gii.ie

GROUNDWATER MONITORING

Hackettstown, Skerries

BOREHOLE	DATE	TIME	GROUNDWATER (m BGL)	Comments
BH03	20/05/2020	12:54	0.97	Dry weather for several weeks previous to monitoring
BH05	20/05/2020	13:45	2.66	
BH07	20/05/2020	12:46	0.97	
RC09	20/05/2020	14:15	5.15	